# UNITED STATES DEPARTMENT OF THE INTERIOR BUREAU OF RECLAMATION

BUREAU OF RECLAMATION HYDRAULIC LABORATORY

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HYDRAULIC MODEL STUDIES OF THE ISLAND
BEND CONTROL GATE - EUCUMBENE-SNOWY PROJECT
SNOWY MOUNTAINS HYDRO-ELECTRIC AUTHORITY
AUSTRALIA

Hydraulic Laboratory Report No. Hyd-462

## DIVISION OF ENGINEERING LABORATORIES



COMMISSIONER'S OFFICE DENVER, COLORADO

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#### ACKNOWLEDGMENT

The model studies discussed in this report were conducted in the Hydraulic Laboratory of the Bureau of Reclamation in Denver during the last half of 1958 and the first half of 1959. During and previous to this time, many conferences were held between members of the Canals Branch, the Mechanical Branch, and the Hydraulic Laboratory Branch of the Bureau of Reclamation and representatives of the Snowy Mountains Hydro-Electric Authority, to discuss original design alternatives, later test results, and final structural and hydraulic requirements. The recommended design for the control structure evolved from these conferences and model studies is therefore the result of the thoughts and work of many people.

A number of Australian engineers associated with the Bureau for a training and observation period assisted in these model studies. The efforts and cooperation of Messrs. Rodney Whitfield, Allen Peet, Colin Kilmartin, Ado Kadak, Daniel Linsten, Christopher Goodall, and Robert Watt were greatly appreciated.

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Subject: Hydraulic model studies of the Island Bend control gate-Eucumbene-Snowy Project--Snowy Mountains Hydro-Electric Authority--Australia

#### PURPOSE

The purpose of the studies was to develop a trouble-free control gate capable of regulating flows ranging from 50 to 7,000 cfs at head differentials up to 400 feet. The gate is used in a tunnel system where submerged flow conditions prevail.

#### CONCLUSIONS

- 1. A gate to satisfy the severe operating requirements, particularly the very low rates of flow under the very highest head differentials, can be obtained with a 9-foot-wide by 16-foot-high structure with weir-like restriction plates placed normal to the flow and flush with the downstream face of the leaf to reduce the effective gate width at small openings (Figures 13 and 20).
- 2. Severe cavitation damage can occur to a structure operating under such high heads. To overcome this danger, an enlarged tunnel section was provided downstream from the gate to form a pool for the high velocity jet to enter. By making the downstream frame short, providing reentrant flow into critical areas, and moving the downstream boundaries outward away from the cavity collapse zones (Figures 4 and 20), cavitation damage to the structure is negated.
- 3. Corner illers to reduce the passage width near the gate bottom (Figure 9) were unsatisfactory due to the occurrence of severely negative (subatmospheric) pressures on the surfaces. Cavitation on these surfaces would have been inevitable.
- 4. Weir-like restriction plates placed normal to the flow and flush with the downstream face of the leaf (Figure 10) provided good pressure and flow conditions. The flow, after leaving these plates, formed its

own boundaries within the fluid. The space behind the plates allowed reentrant flow between the jet and the walls and relieved negative pressures. Plates with convex flow edges (Figure 100) provided best results.

- 5. The floor of the downstream gate body must be sloped downward to obtain a more rapid degree of conduit expansion. A vertical 12-inch downward step at the downstream face of the restriction plates, followed by a 45° downslope to elevation 3503.25 (Figures 11 and 20), gave good results.
- 6. Ideally, the enlarged tunnel section should be placed as close as possible to the gate leaf to prevent cavitation damage and to obtain the best recirculation around the discharging jet. However, structural considerations required that the downstream conduit width be the same as the upstream width for a sufficient distance to resist the gate leaf thrust. A good hydraulic and structural balance was obtained with 7-foot-long parallel walls 9 feet apart, followed by 11-1/2-inch outward offsets and 18° outward flares (Figure 13).
- 7. Outwardly offset downstream slot corners, followed by curved walls to return the passage width to normal, did not improve the slot and wall pressures in this gate. A square-cornered rectangular slot (Figure 12) performed as well as the offset ones and offered design and construction economies. The back pressure acting on this structure was the main factor permitting the selection of the square-cornered slot design.
- 8. The usual open-beam shape of gate leaf bottom for this downstream seal, roller-mounted gate leaf was unsatisfactory due to poor local pressure conditions. Much better performance was obtained with an elliptically shaped bottom (Figure 13).
- 9. The position and shape of the gate leaf seal that seats against the upstream face of the restriction plates greatly affected the pressures near the leaf bottom. Satisfactory pressures are expected with the seal placed as low as possible on the downstream face of the leaf and projecting only 9/32 inch beyond the seal bar (Figure 10F).
- 10. The pressure and flow conditions for the gate should be entirely satisfactory for all operation with the Island Bend and Geehi Pond water surface elevations within their normal ranges. Even at the extreme but unlikely condition of Island Bend Pond completely full and Geehi Pond completely empty, but with a 30-foot back pressure, the operation will be acceptable. There will be a slight tendency for light, sporadic, local cavitation near the flow surfaces.

- 11. When Geehi Pond is empty and low flows are passed to it through the Island Bend gate, it will be necessary to check Geehi gate to maintain a back pressure of at least 30 feet on the Island Bend gate, measured from the upstream center line.
- 12. The Island Bend gate may be operated at any gate opening greater than 6 inches under any applicable head conditions for any length of time. The gate opening limitation will pose no handicap on the gate or on the operation of the overall tunnel and reservoir system because the minimum specified discharge of 50 cfs at the greatest possible head differential is expected to be obtained with an opening slightly greater than 6 inches.
- 13. Stainless steel plates are needed in the Island Bend gate on the parallel 7-foot-long walls downstream from the slots to provide and maintain smooth, well alined, corrosion- and erosion-resistant surfaces. Such surfaces are mandatory on these parts of a structure that operates submerged at practically any opening, under very high differential heads, for very long periods of time, and where minimum maintenance is important due to the difficulties, loss of time, and heavy expenditures necessary in gaining access to the gate.
- 14. A measurement of downstream head is needed in addition to knowledge of gate opening and upstream head to set given discharges through the Island Bend gate. This measurement can be made with a float well in the gate shaft, the well being connected to an opening in the side of the 28-foot-diameter enlarged tunnel 100 feet downstream from the gate.
- 15. The downpull forces acting on the gate due to pressure decreases on the bottom surface when flow passes beneath the leaf will reach a maximum of about 442,000 pounds (Figure 19A). This load will occur at a gate opening of about 60 percent and is for the severe condition of Island Bend Pond full and Geehi Pond empty. Less downpull occurs as Geehi Pond fills.
- 16. Identical gate leaves and structures can be used at Island Bend and Geehi control stations. This provides certain design economies and flexibilities and will insure having a gate at Geehi that will satisfactorily handle the very severe operating conditions that can be imposed on it.
- 17. Tests made without restriction plates in the gate body, thus representing the Eucumbene gate, and with Lake Eucumbene considered at minimum operating level and Island Bend Fond full, showed an approximate maximum downpull of 429,000 pounds (Figure 19B).

18. Tests made without restriction plates and with the tunnel empty downstream from the Eucumbene gate, as could prevail during initial filling of the Eucumbene Tunnel if attempted from Island Bend, showed a maximum downpull force of approximately 1,070,000 pounds (Figure 19C).

#### INTRODUCTION

The Snowy Mountains Hydro-Electric Scheme in southeastern Australia is a comprehensive engineering and construction program for collecting waters from the seaward and landward sides of the Great Dividing Range and using them for irrigation and power generation (Figures 1 and 2). The scheme is under the direction of the Snowy Mountains Hydro-Electric Authority. The Eucumbene-Snowy-Geehi Diversion System is a part of this scheme and will transfer Snowy River water into M-6 (later changed to Murray 1) and subsequent power stations, or into the main storage in Lake Eucumbene, or from lake Eucumbene directly into the Murray-series power stations (Figure 2). The Island Bend control structure is a part of the Eucumbene-Snowy-Geehi Diversion plan (Figure 3). Its function is to regulate the flows released from Island Bend Reservoir into the tunnel system, while maintaining the Island Bend inlet shaft and tunnel full of water that air is not entrained and carried into the system.

The question of whether or not to allow air to enter the tunnel system was carefully considered. Attention was particularly given to problems that would arise if air were present. It was believed that pockets or accumulations of air would build up along the crown of the gradually sloped tunnels, particularly in the unlined sections, and would move intermittently along with the flow. Upon reaching the tunnel outlets, which are normally deeply submerged at the reservoirs, the air pockets would quickly vent themselves, and water would rush into the vacated space. Heavy shock loads and water hammer would then occur as the waters moving into the vacated areas collided. These shocks would be damaging to the structures and undesirable from an operational standpoint. In addition, violent and undesirable boiling and wave action would occur in the ponds.

Vents placed at strategic locations could alleviate these difficulties by providing controlled release of the air. The problem would then resolve itself into obtaining satisfactory separation of the air from the water and providing suitably located collectors and vents to exhaust it. Preliminary studies of methods to achieve this controlled removal of air from the tunnels showed that several unknowns were involved. Foremost of these was the rate of rise of various-sized bubbles in the turbulent flow in the tunnels. Because of the unknowns, it was not possible, on the basis of available data, to say what type, size, and placement of air separation chambers and vents would be needed to release the entrained air from the water and vent it to the

atmosphere. It was finally decided best to avoid these problems by adopting a design which prevented the entrance of air by making the inlet flow full at all times. The entrance (Figures 4 and 5) was therefore made somewhat similar to the inlet for the junction shaft in the Eucumbene-Tumut Tunnel.1/ The regulation in the Island Bend structure will be accomplished by a gate in the horizontal section of tunnel between the Island Bend intake and the connection with the Eucumbene-Snowy and Snowy-Geehi Tunnels. This gate will at all times be operated to keep the intake structure flowing full so that no air is entrained and carried into the system.

The operating conditions imposed on the control structure are severe. The maximum flow will be about 6,400 cfs. The minimum is about 50 cfs. Prior to the initial filling, and possibly on rare occasions thereafter, Geehi Pond may be completely empty. During the early stages of filling Geehi Pond, free flow discharge conditions can prevail at Island Bend gate because the gate invert is above the minimum water surface level of Geehi Pond (Figure 4). No operational problems are anticipated with the free discharge operation if the gate is operated to fill the pond.

As the water level rises in Geehi Pond, the tunnel will fill and submerged discharge conditions will prevail at Island Bend gate. This type of operation greatly alters the flow and pressure conditions at the gate and produces the most severe operating conditions. A head differential of 390 feet can occur across the gate when the 28-foot-diameter conduit is just full. Frictional losses in the tunnels will add back pressure as the rate of flow increases. Computed frictional losses for the largest and smallest probable roughness values and the back pressures on the center line of the Island Bend gate with minimum pond at Geehi are shown in Figure 6B. As the pond continues to fill to reach normal operating levels, and/or as the discharge increases, the back pressure increases while the head differential decreases. Progressively less severe operating conditions result. A similar but less severe situation exists with flows to the Eucumbene-Snowy Tunnel (Figure 6A).

A number of different gate designs were considered for the Island Bend control structure. One included placing 3 moderately sized gates side by side with only 1 operating when releases were small. Another plan included a large single gate with a small gate built into the main leaf. These and other designs were eventually eliminated from consideration and a single 9-foot-wide by 16-foot-high gate was selected. Extremely small gate openings during releases of very small flows were rendered unnecessary by decreasing the width of the lower past of the gate

<sup>1/</sup> Numbers refer to References at end of report.

body. This reduction in passage width was carried to the point where openings of 6 or more inches could be set for releasing the minimum anticipated discharge of 50 cfs.

In systems where high velocity flows enter a pool or filled conduit under moderate or low back pressures, cavitation can occur.2/3/
This cavitation takes place within the low pressure cores of rapidly whirling vortices created by shearing between the high velocity jet and the surrounding water. As long as the vortex action is severe and the pressure regime is moderate or low, the inception, formation, and decay of vapor cavities continue. If these multitudes of vapor pockets collapse on or near the boundary walls, damage of serious proportions can occur. It is therefore imperative, for long structural life, that the boundary surfaces of such a structure be placed well away from areas where this inevitable cavitation collapse will take place.

Design data, unpublished at the time of this writing, were available in the Bureau of Reclamation laboratories to help in determining the conduit arrangement for the Island Bend gate. Studies using a 400-foot head differential across a partly opened, 3-inch, 300-pound gate valve showed that no damage would occur on the downstream conduit provided its diameter were 1.75 or more times the nominal gate body diameter and the back pressure in the conduit were 20 or more feet. A review of the Island Bend installation showed that it would almost always operate submerged and that its back pressure could become as low as 20 to 30 feet. Further decreases in back pressure with the possibility of undesirable shifting between submerged and free discharge flow conditions would be prevented by throttling the Geehi control gate located a short distance downstream. The severest Island Bend operating condition was therefore submerged flow regulation with a 410-foot upstream head and about a 30-foot downstream head. On the basis of the laboratory data, the downstream conduit diameter, D, was made 1.75 times the 16-foot diameter of the upstream conduit, or 28 feet. The length, based on the same data, was made 5D, or 140 feet, to provide ample room for flow redistribution before the water reentered the 16-foot-diameter tunnel farther downstream (Figure 4). Later tests showed this design to be satisfactory.

Several hydraulic problems were apparent in the design of this unusual control structure. Cavitation and cavitation-erosion were an everpresent threat and could be encountered not only in the mixing zone of the jet but on the walls, gate slots, leaf, or other parts of the structure. Positive means for controlling the cavitation had to be found if satisfactory service life and dependability were to be achieved. In this respect, there was uncertainty about the shape of the restrictions to be used within the gate body to narrow it near the invert. Also, the shape of the gate leaf bottom and the style and placement of the seals were uncertain. Furthermore, the shape of the abrupt expansion

downstream from the point of control in the structure had to be determined and balanced against a sound structural design capable of withstanding the terrific thrust loads on the leaf. To determine what these shapes and designs should be for the overall control structure, hydraulic model studies were made. Descriptions of the test facilities used in the studies, and discussions of the results obtained, are presented in this report.

#### THE MODELS

All preliminary studies were made with air because air model tests are convenient, fast, inexpensive, and reliable. The air facilities used for the tests consisted of a centrifugal blower which drew air in through a 12-inch-diameter inlet pipe and forced it out through a 10-inch pipe to the gate section (Figure 7A). After the flow passed through the gate section, it entered an enlarged conduit similar to the enlarged downstream tunnel proposed for the prototype structure. The air then discharged freely back into the atmosphere. The rate of air flow was measured by appropriately sized flat plate orifices located on the entrance to the 12-inch blower inlet. The flow velocities within the gate were maintained below 300 feet per second so that the compressibility of the air would be negligible. The pressure measurements were obtained with piezometers and water-filled manometers read to the nearest 0.01 of an inch. In cases where pressure fluctuations were small or moderate, the average pressures were recorded. In other cases, where fluctuations were severe, the most negative pressures were recorded in addition to the average ones.

The gate section for the 1:19.2 scale air model was constructed of wood, light gage sheet metal, and transparent sheet plastic (Figure 7B). The design shown consisted of the 5.6-inch-wide by 10.0-inch-high gate section, the 2.77-inch-thick gate leaf, and the 5.6-inch-wide by 14.25-inch-high downstream frame 8.13 inches long. The enlarged downstream conduit was represented by a 20.5-inch-diameter sheet metal pipe. Piezometers were provided on the flow surfaces believed to be critical.

The final model tests, including calibrations and downpull measurements, were made with a 1:24 scale hydraulic model (Figures 8A and 13). The water facilities for making the tests consisted of the permanent laboratory water supply system, the upstream pipeline or tunnel section, the gate itself, the downstream pipeline or tunnel section, the back pressure regulating valve, and the waste line to the laboratory reservoir. Water was supplied to the model by a 12-inch centrifugal pump, and the rate of flow was measured by calibrated Venturi meters within the permanent laboratory supply system. Water approached the gate through an 8-inch-diameter pipe 12.5 feet long. A transition similar to the one proposed

for the field structure carried the flow into the 4.5-inch-wide by 8.0-inch-high gate. A 14-inch-diameter pipe represented the 28-foot-diameter enlarged conduit downstream, and a 12-inch gate valve regulated the back pressure. The water returned to the laboratory storage reservoir for recirculation.

The gate section of the hydraulic model was constructed of brass plate and 1/2-inch-thick transparent plastic (Figures 8B and 13). It consisted of the 4.5- by 8.0-inch upstream frame, the 2.22-inch-thick gate leaf, and the downstream gate body with a downwardly flared floor. Piezometers were provided at areas found to be critical in the air model studies so that final pressure measurements could be made. Care was taken to make the hydraulic model strong and capable of withstanding relatively high heads. This was necessary because reasonably high velocities were required to obtain Reynolds number values high enough for good model-prototype similitude. 4

Pressure measurements were read on water-filled, single leg manometers, mercury-filled single and double legged manometers, and in some cases, from pressure cell recording charts. The charts were obtained at those piezometer stations where the pressures were particularly low and/or where the pressures fluctuated greatly.

#### INVESTIGATION

## Studies for Narrowing Lower Part of Gate Body--Air Model

#### Triangular Corner Fillers

The first attempts to narrow the lower portion of the 9-foot 5-1/2-inchwide preliminary gate body were made by inserting corner fillers in the passage (Figure 9). These fillers were triangular in cross section and extended the full 12 feet from the gate slots to the abrupt expansion into the 28-foot-diameter tunnel. The upstream ends of the fillers were shaped to produce elliptical surfaces. The flow passage formed at small gate openings by the gate leaf bottom, the sloping walls of the specially shaped fillers, and the floor was trapezoidal in cross section and of small area.

The major and minor axes of the first elliptical section were 3.0 and 0.96 feet (prototype), respectively, forming a 1:3.13 ellipse (No. 1, Figure 9B). The ellipses lay in a plane parallel to the gate floor and the fillers were 3 feet apart at the floor. The spring lines, or lines where the elliptical curvature started at the vertical upstream faces of the fillers, were 4 feet 11 inches apart at the floor. The design therefore produced a downstream gate body 4 feet 11 inches wide at the

floor, gradually widening to the full 9-foot 5-1/2-inch passage width at a height of 5.45 feet. The full height of the fillers at and beyond the end of the elliptical section was 6 feet 5-1/2 inches.

Severely negative pressures occurred on both the curved and straight surfaces of the fillers. The pressures were particularly low at the piezometer located 10 inches downstream from the start of the fillers and 10 inches above the floor at a 13 percent gate opening. Apparently the 13 percent gate position placed severe flow contractions directly over the piezometer station.

A second shape was investigated using an ellipse with prototype major and minor axes of 5.0 and 1.0 feet, respectively (1:5 ellipse) (No. 2, Figure 9B). These fillers were larger than the first ones, and the width between the parallel portions was reduced to 1.5 feet at the floor. The spring lines were 3 feet 6 inches apart at the floor and met the side walls 5 feet 11-1/2 inches above the floor. The overall height was 7 feet 11-5/8 inches.

Low pressures were again encountered on the curved surfaces near the gate leaf, and to a lesser extent on the straight sides of the fillers. Pressures measured on the floor near the intersection with the fillers showed moderately negative pressures. The gate position had considerable effect upon the pressures near the spring lines of the ellipses, and the lowest pressures on any particular piezometers occurred when the leaf bottom was about level with the piezometer openings.

In the above two designs, the ellipses were laid out in planes parallel to the floor. Actually, the flow tends to travel normal to the spring line. It was thus more appropriate to lay out the ellipses in a plane normal to the spring line and hence normal to the slope of the filler surfaces. A third bellmouth was made up in this manner with major and minor axes of 6.25 and 1.25 feet prototype (1:5 ellipse) (No. 3, Figure 9B). The spring lines were 3 feet 3-1/4 inches apart at the floor and met the side walls at a height of 6 feet 2-1/4 inches. The total filler height was 8 feet 11-1/2 inches.

The pressure conditions were better with this design, but serious negative pressures still occurred near the leaf. This suggested that even more gentle curvature was needed, with larger major and minor axes for the ellipses. However, attempting to maintain the spring lines close enough together to properly narrow the passage, and yet attempting to use fillers with larger ellipses, was inconsistent and impracticable. No further increase in minor axis was therefore reasonable, and no further tests were made with the triangular fillers.

## Rectangular Corner Fillers

It was believed that part of the reason for severe negative pressures on the upstream parts of the fillers was that an acute angle was formed by the horizontal bottom of the gate leaf and the sloping walls of the triangular fillers. To increase this angle as much as practicable, fillers with vertical sides were tried in the model (Figure 9C). Elliptical surfaces with 6.25- and 1.25-foot (prototype) axes were formed on the sides and tops of the fillers. The vertical spring lines were 4 feet 4-3/4 inches apart, and the horizontal spring lines at the filler tops were 5 feet 1-3/4 inches above the floor. The full filler height was 6 feet 4-3/4 inches. The distance between the parallel portions was 1 foot 7-1/2 inches.

Somewhat better pressure conditions occurred with the rectangular fillers when the gate leaf was below the top spring lines. However, when the leaf was raised to just allow flow over the top of the fillers, negative pressures occurred on the top and on the corner surfaces that connected the vertical and horizontal ellipses. Also, small increases in gate opening in this range produced large changes in the quantity of water released. This was undesirable because it made the setting of small discharge changes difficult. At large gate openings, the pressures on the fillers were satisfactory.

## Triangular Restriction Plates

The difficulties encountered in using solid corner fillers to reduce the passage width led to an entirely different concept. It was reasoned that if the water were allowed to form its own flow boundaries after leaving the control section at the leaf, and no structural surfaces were near these flow boundaries, there would be less difficulty with negative pressures. Heavy flat triangular weir-like plates were therefore fastened vertically in the gate body normal to the passage and flush with the downstream face of the gate leaf (Figure 10). These restriction plates represented the front surfaces of the previous fillers and narrowed the passage in the same manner. After the flow left the edges or spring lines of the plates (Figure 10E), it was free to take its natural flow path. The space behind the plates allowed reentrant flow between the jet and the gate body to relieve negative pressures. It, in effect, provided an abrupt expansion downstream from the leaf.

The first tests were made with triangular plates 4 feet 5-3/4 inches wide and 8 feet 11-1/2 inches high, prototype (Figure 10B). This left a passage width at the floor of 6 inches. Better pressure conditions were encountered on the floor and walls downstream from the plates than on the boundary surfaces with any previous designs. Tests were continued with variations in width and height of the plates to obtain a design that

combined acceptable pressure conditions, ample flow recirculation behind the restrictions, sufficient reduction in passage width for controlling small flows, and access room for maintenance men and equipment. At the same time, the overall width of the downstream gate body was reduced to an even 9 feet. Best results were obtained with triangular plates having a bottom width of 4 feet and a height of 6 feet.

## Curved Edged Restriction Plates

Continued studies with restriction plates showed that better performance could be obtained if the flow edges or spring lines were convex instead of straight (Figure 10C). The convex shape restricted the flow passage a little more and provided more room for reentry of water between the jet and walls. The results were that the pressures on the floor and walls were better, and the flow was more stable. Best performance was produced by sharply curved plates, but the difficulty of supporting such plates in the prototype structure made their use unwise. An excellent compromise was achieved with moderately curved plates, and this design is recommended for prototype use (Figure 10C).

## Leaf Bottom Shape and Bottom Downstream Seal--Air Model

An important factor learned from the studies with corner fillers and restriction plates was that the shape of the gate leaf bottom and the placement of the downstream seal had a marked effect upon local pressure conditions. The downstream bottom seal, which is necessary for sealing along the face of the plates, was first placed well above the gate leaf bottom (Figure 10D). This produced a restricted space between the rear of the gate leaf and the face of the plates, and local high velocity flows occurred. These flows caused extremely low pressures and were undesirable. The difficulty was largely overcome by moving the seal close to the bottom of the gate and by keeping the extension of the seal beyond the clamping bars to a minimum (Figures 10F and 20). Thus, no appreciable passage remained and no large flows were present between the leaf and the plates.

The bottom geometry of the initial leaf was of the type frequently used with fixed-wheel or roller train—gates (Figure 10D). It was formed by the undersurface of the large I-beam used horizontally at the bottom of the leaf, the plate extensions at the downstream face, and vertical stiffener plates from the beam to the extension. The bottom seal, which rests on the floor at gate closure, was carried on the extension plates. As initially proposed, the seal extended downward beyond the plates about 3/8 of an inch, and there was a sharp corner at the bottom upstream edge of the plate. Tests showed extreme negative pressures on this leaf bottom. Minor revisions like beveling and rounding the upstream bottom corner and reducing the extension of the seal beyond the plates improved the pressure conditions only moderately.

A major change in bottom geometry greatly improved the pressures and flow conditions. This change consisted of extending an elliptically shaped steel plate from the front face to the bottom downstream edge of the leaf (Figures 10A, 10F, and 20). The elliptical shape is recommended for the prototype leaf (Figure 20) and was used for all subsequent model tests.

## Flared Floor and Walls--Air Model

## Floor Downstream from Gate Leaf

The basic principle used in the design of the gate was that of providing an enlarged section for the high velocity flow to enter. For this principle to be workable, the enlarged section must be large enough to give good flow circulation around the jet. Ideally, this enlargement would be achieved just downstream from the gate control section. However, for structural reasons, it was necessary to continue the gate body, or conduit, for an appreciable distance downstream from the slots so that the slots would be capable of withstanding the thrust loads of the leaf. This extended conduit restricted the flow recirculation and caused a general lowering of the pressures and some instability in the jet.

Increased expansion, or enlargement of the passage, was obtained by changing the alinement of the downstream floor (Figure 11). To accomplish this in the model, the downstream frame was modified by dropping the horizontal floor to the level where the bottom corners just intersected the walls of the 28-foot-diameter tunnel. This represented the greatest permissible drop in floor elevation of 6.8 feet, prototype. Tests were then made with horizontal fillers inserted to represent level floors 1.7 and 3.4 feet below the upstream floor. The pressures and flows became progressively better as the floor was lowered, and the best conditions were obtained with the lowest possible floor level of 6.8 feet below the upstream frame (Figure 11A). An appreciable amount of turbulence and pressure fluctuation remained in the system.

Other floor fillers were tested. One provided a 45° slope from the upstream floor level to the fully lowered level floor downstream (Figure 11B). Good flow and pressure conditions resulted. The second filler provided a more gradual slope that started from the upstream floor level and reached the fully lowered floor at the downstream end of the 13-foot-long body section. This design was less satisfactory than the 45° slope.

Further tests were made to determine the pressure conditions on the 45° slope just downstream from its intersection with the upstream floor. Severe negative pressures occurred at and near this change of alinement

unless an appreciable vertical step, or offset, was provided (Figure 11B). A step of 3/4-inch prototype was first tested but was found entirely inadequate. The step, or offset, was eventually increased to 12 inches prototype, and this amount of offset represented a satisfactory compromise between structural and hydraulic considerations in the system. The 45° slope, together with the 12-inch vertical offset, is recommended for prototype use.

### Walls Downstream from Gate Leaf

The 9-foot-wide by 13-foot-long walls downstream from the leaf still formed too long and narrow a passage to allow adequate flow recirculation around the main jet. Detailed stress studies showed that it would be possible to flare the downstream 6-foot sections of the 13-foot-long walls while still retaining adequate strength to withstand the extreme thrust loads of the leaf. Tests were made with these walls flared 30°, 20°, 10°, 0°, and -10° relative to the passage center line (Figure 11C) and using the 45° sloping floor with a 12-inch vertical step at the restriction plates. The best pressure and stability conditions occurred with the maximum flare, and progressively poorer results occurred as the walls were moved inward. The worst conditions occurred with the walls converging at the -10° flare. At the maximum flare of 30°, the pressures measured on the 7-foot-long parallel sections of wall, and at critical locations on the flaring walls, were satisfactory for all expected flow conditions.

## Shape at Downstream Slot Corners--Air Model

Preliminary tests using an early gate leaf design showed negative pressures on the walls just downstream from the square slot corners (Figure 12). Attempts were made to reduce these negative pressures by providing outwardly offset slot corners, followed by long radius curves that returned the passage width to that of the upstream frame (Figure 12B). Outward offsets at the slot corners of 1.5 and 3.0 inches, prototype, were tested. The curves from the slot corners to the parallel walls downstream were made with a 30-foot radius. This curved portion of wall extended vertically from the ceiling to the top of the restriction plates. The full square slot shape was used from the top of the plates down to the floor. A ledge was present at the top of each restriction plate (Figure 12A).

Tests made using the final gate leaf design showed that the pressure conditions with the two offset slot designs were about the same as with the original square-shaped slot (Figure 12C). When the gate bottom was at or slightly below the ledge, negative pressures occurred above the ledge and on the slot corner just beneath the leaf bottom. These pressures, which were calculated for the very minimum back pressure

conditions on the gate, were more negative than desired. They should nevertheless not be detrimental for limited operation at minimum back pressures, and should be entirely satisfactory for all operation at the greater back pressures that occur under normal operating conditions. There appeared to be no advantage in using the offset slot corners in place of the in line square ones, and the square ones are recommended for prototype use.

It is to be emphasized that a most important factor in these slot pressures being satisfactory is that back pressure occurs on the gate. Pressure reductions in local areas are held above critical values by this back pressure, thus maintaining reasonable pressure values. Tunnel friction provides a great deal of back pressure at large flows, and additional back pressure at all flows will normally be present due to ponding at Geehi. If it were not for the back pressures expected to occur, these slot designs could not be used for high velocity releases under submerged conditions.

## Recommended Design-Hydraulic Model

#### Final Model

An hydraulic model incorporating the best features obtained through the previous air model studies was constructed on a 1:24 scale (Figure 13). The moderately curved restriction plates were used. The conduit floor stepped vertically downward the equivalent of 12 inches at the downstream face of the plates and then continued downward on a 45° slope to the short horizontal section at elevation 3503.25. The walls of the gate were 9 feet apart and parallel for 7 feet beyond the gate slots. The walls then stepped outward 11-1/2 inches on each side and continued with outward flares until intersecting the 28-foot-diameter tunnel. Flare angles of 30° and 18° relative to the conduit center line were investigated. The roof of the gate continued horizontally 7 feet from the slots and then sloped upward. The gate slots were rectangular with the upstream and downstream corners in line. The gate leaf incorporated the elliptical bottom, the enclosure boxes for the roller trains, and a downstream seal.

Piezometers were included in the model in areas likely to be critical so the pressure conditions could be observed. These pressures were read with single leg water manometers and a mercury gage. In cases where the pressures were negative or fluctuating widely, the measurements were also made with a pressure cell and electronic recording equipment.

### Gate Performance

The first tests were made with 30° side wall flares. All pressure conditions were satisfactory at appropriate settings of upstream and downstream heads.

Tests were then made with 18° flares because the lesser flare angle permitted a more desirable construction with greater strength for resisting thrust loads on the leaf. The pressure conditions and flow stability were not as good as with 30° flares, but were nevertheless within reasonable limits for even the most severe operating conditions. The 18° flares were thus found satisfactory for prototype use, and all subsequent tests were made with them.

The pressures obtained from the model are presented in two forms. The

data were first converted into pressure factors, PF =  $\frac{h_x - h_2}{H_t - h_2}$ , where

h, is pressure at the piezometers, ho is the pressure head in the conduit downstream from the gate, and Ht is the total head at the reference station just upstream from the gate. The pressure factors for Island Bend gate are shown in Figure 14A. By using these pressure factors and the pressure conditions expected to occur upstream and downstream from the gate for the most severe operating condition (Island Bend full and Geehi empty), the pressures at critical piezometers were computed (Figure 14A). The tunnel pressures used in the calculations are shown in Figure 15B. Tunnel pressures for more nearly normal operation with Geehi Pond full are shown in Figure 15A. With the latter operating conditions, all pressures on the gate structure will be strongly positive. By use of the pressure factor data, individual pressures can be computed for any desired operating condition. The coefficient of discharge for various gate openings is shown in Figure 16. Prototype discharges may be estimated, when the upstream and downstream pressure heads are known, by using the nomograph in Figure 17.

Pressure cell records of the most severely negative and/or the most fluctuating pressures (Figure 18) illustrate the most critical prototype conditions to be encountered if the gate is throttled with Geehi Pond empty and Island Bend Reservoir full. The worst conditions occur at gate openings between 25 and 45 percent. The most critical locations were between the restriction plates on the vertical step in the floor (Piezometers 21 and 22) and on the walls downstream from the plates (Piezometers 15 and 16). In cases of small discharge where friction losses would be insufficient to maintain back pressure at Island Bend, it was assumed that Geehi control gate would be throttled to maintain the tunnel full with a 30-foot back pressure above the Island Bend gate upstream center line (Figure 15B). It will be noted that at these most

severe operating conditions, the instantaneous pressures occasionally and briefly extend downward into the cavitation range (Figure 18). The fact that these pressure excursions are brief, occur only under this unlikely operating condition, and are on areas where cavitation collapse is unlikely due to the flow patterns makes this problem relatively minor. As Geehi Pond fills, the back pressure at Island Bend increases and the head differentials across the gate drop. The operating conditions thus become progressively less severe as normal operating conditions are approached.

At gate openings below 25 percent, all pressures are steady and well positive and the gate performance is smooth at all applicable flow conditions. At gate openings between 25 and 45 percent, random pressure fluctuations occur, apparently due to the discharging jet swinging erratically in the downstream passage. The fluctuations are most severe when Island Bend Reservoir is full and Geehi Pond is completely empty. This operating condition is, at best, only a remote possibility, but if it does occur, consideration should be given to operating the gate at openings either smaller than 25 percent or larger than 45 percent. As the back pressure rises, due to filling Geehi Pond, the pressure fluctuations decrease rapidly. At gate openings larger than 45 percent, even with Geehi Pond empty, the pressure fluctuations decrease, and good operation is expected.

## Hydraulic Downpull

When flow occurs under the gate leaf, pressure head is converted into velocity head and the pressures on the leaf bottom become low relative to the pressures on the leaf top. This pressure difference, applied over the cross-sectional area of the leaf, produces a downward force which must be considered in designing the gate-lifting mechanisms, stems, and supporting structures.

The downpull forces were determined by pressures measured with piezometers in the leaf bottom and in the gate bonnet. The averages of these pressures were applied to appropriate areas on the leaf, and the overall approximate downpull was then computed. No measurements of stem loads were made because the model was not built to permit direct measurements. These direct measurements require a nearly frictionless suspension for the leaf within the gate body, a condition not easily obtained in a model of a very high head gate.

The pressures in the bonnet and on the bottom of the Island Bend gate leaf are shown in Figure 14A, Piezometers 44 through 59. These pressures are for the severe condition of Island Bend Pond full and Geehi Pond empty. The approximate maximum downpull, based on the difference between the pressure acting downward on the top beam of the gate and the

arithmetic average of the applicable pressures in each piezometer row acting upward on the leaf bottom, and the areas affected, is shown in Figure 19A. The greatest force occurs at about a 60 percent gate opening and reaches 442,000 pounds.

Tests were also made with the restriction plates removed from the body and with the severest head conditions that would apply to the Eucumbene gate. This test was made to determine the forces to be expected if emergency or other closures are made with the Eucumbene gate. The very different discharge characteristics of the design without restriction plates produced a very different downpull curve (Figure 19B). Relatively low pressures, considering the submergence due to Lake Eucumbene (Figure 14B), occurred on the leaf bottom across the entire passage to produce the maximum downpull of 429,000 pounds at about a 22 percent gate opening. The pressures at the gate slots had an important effect on the curve, being strongly positive at 10 percent and smaller openings and decreasing rapidly as the gate was opened from 10 to 20 percent. At larger openings, with attendant tunnel back pressure increases and hence differential head decreases, the downpull progressively decreased. Pressure factors and discharge coefficients for the above operation are given in Figure 14B.

A last series of downpull tests was made with the restriction plates removed and with free discharge conditions downstream from the gate. This approximates the conditions that would possibly occur during early filling operations in the Eucumbene-Island Bend Tunnel. With an upstream head of 410 feet, cavitation pressures occurred on the leaf bottom at gate openings of 40 percent or more (Figure 14C). The maximum downpull occurred at about a 52 percent gate opening and reached 1,015,000 pounds (Figure 19C). The downpull computations were made using -30 feet for all pressures indicated within the cavitation range.

The pressure data and discharge coefficients for the free discharge operating condition are presented dimensionlessly in Figure 14C. These data, like the other pressure factor data, may be used for a wide range of pressure conditions. It is therefore usable for other operating conditions on this structure and for other structures that are geometrically similar.

### Stainless Steel Liner Plates

The severe operating conditions on the Island Bend gate, and the fact that it is extremely difficult and expensive to reach for maintenance purposes, make it mandatory to provide smooth, well alined, corrosion-and erosion-resistant surfaces wherever high velocity flows will occur in the gate. Typical surfaces in need of special care are found on the

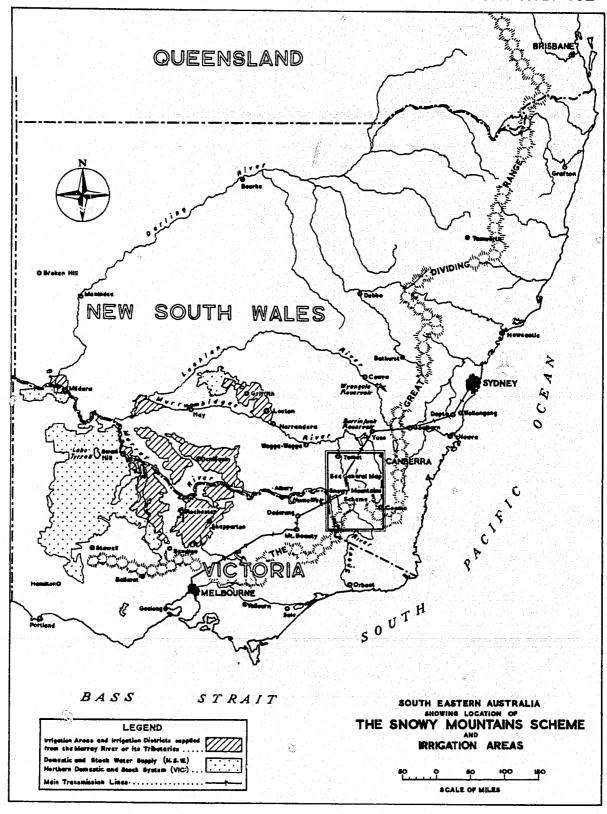
parallel, 7-foot-long walls downstream from the gate slots and on the restriction plates where the downstream gate seal makes contact. It is believed that carefully placed, firmly supported, and accurately alined heavy stainless steel plates will provide the best possible and longest-lived surfaces for these critical areas. Such plates are therefore provided in the design of the prototype gate (Figure 21).

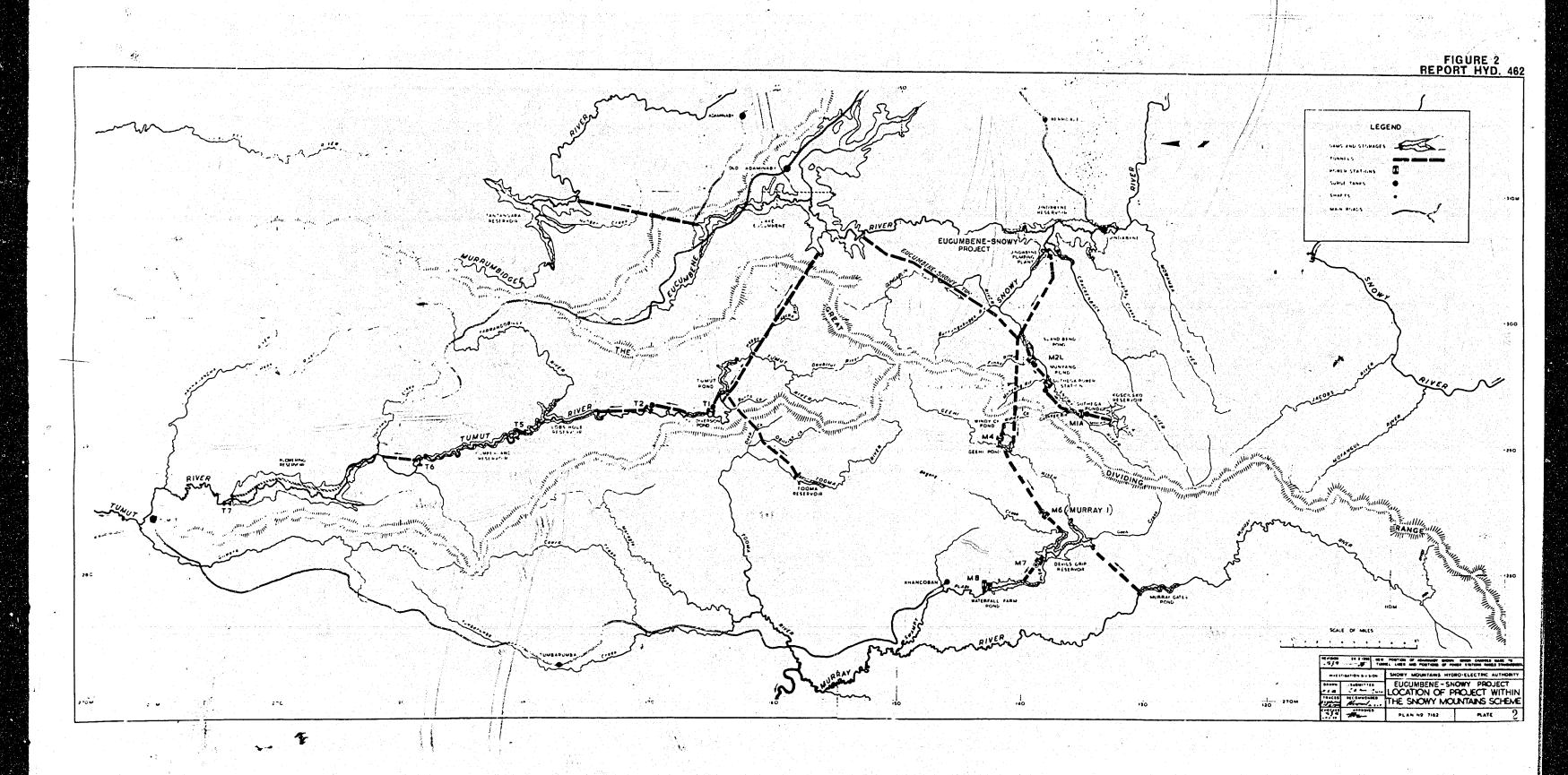
## Measuring Station for Downstream Tunnel Pressure

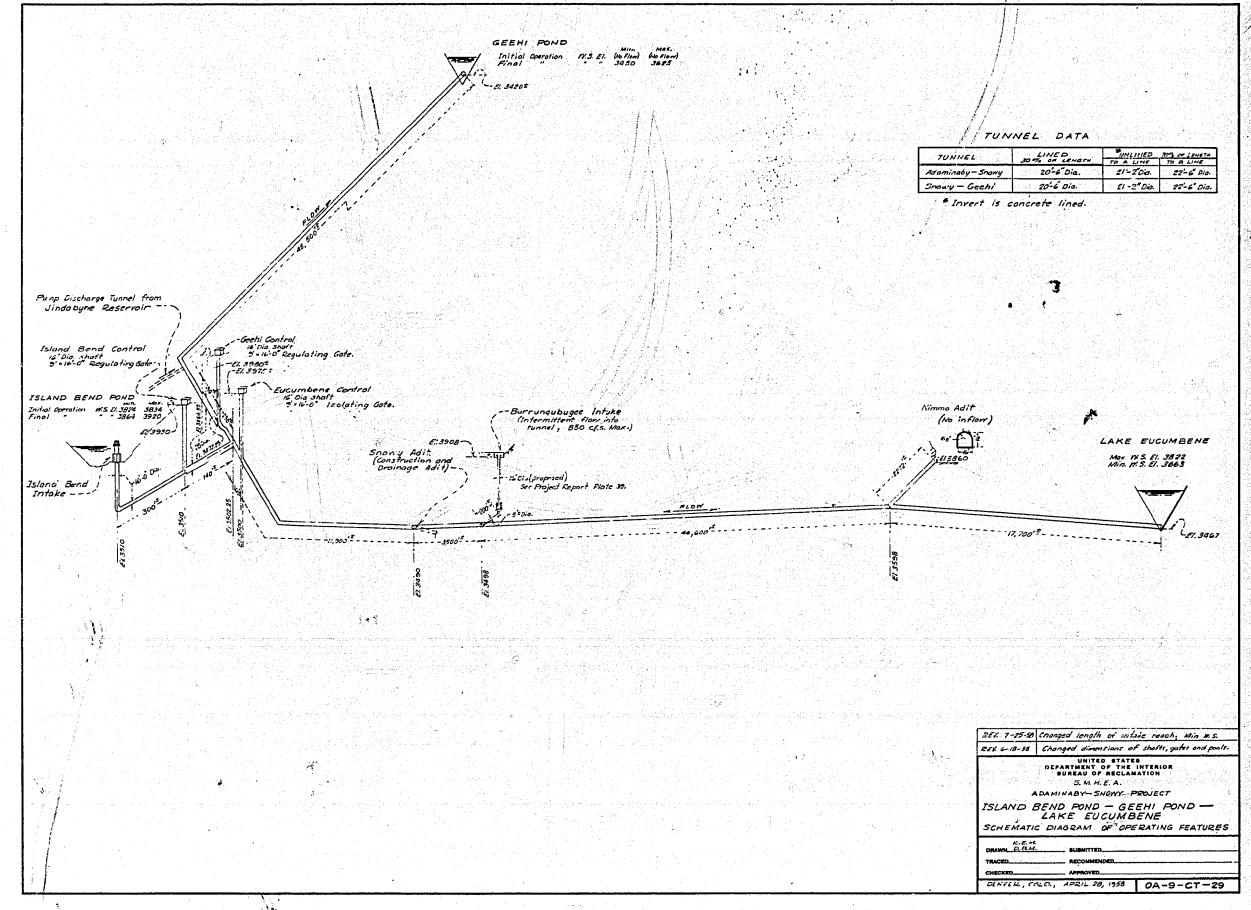
The rate of discharge through the gate will depend not only upon the elevation of Island Bend Pond and the gate opening but also upon the head, or back pressure, in the 28-foot-diameter tunnel downstream. To measure this head, a float well is provided and is connected to the 28-foot-diameter tunnel at a point 100 feet downstream from the Island Bend gate (Figure 4). This distance allows reasonable redistribution of flow downstream from the gate and is far enough upstream from the contraction to the 16-foot tunnel to avoid interference. The opening into the tunnel is placed on the horizontal center line instead of the vertical one to avoid any accumulations of separated air.

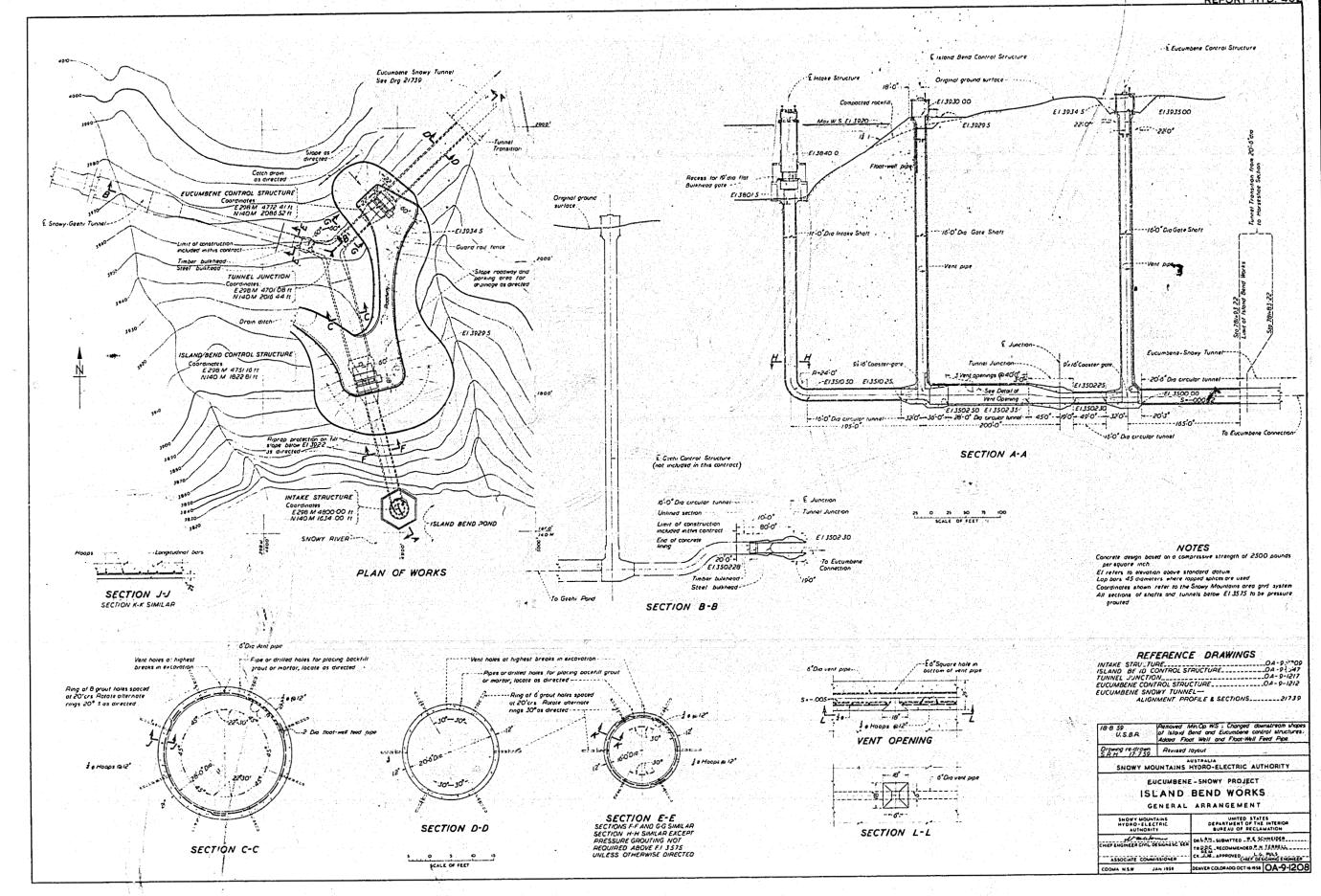
#### REFERENCES

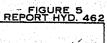
- 1. Schuster, J. C., "Hydraulic Model Studies of the Eucumbene-Tumut Tunnel Junction Shaft for the Australian Snowy Mountains Hydro-Electric Authority," Bureau of Reclamation Report No. Hyd-392, August 31, 1954
- 2. Rouse, Hunter, "Cavitation in the Mixing Zone of a Submerged Jet,"
  La Houille Blanche, January-February 1953, pp 9-19
- Ball, J. W., "Cavitation Characteristics of Gate Valves and Globe Valves Used as Flow Regulators Under Heads up to About 125 Feet," Transactions ASME, August 1957, pp 1275-1283
- 4. Simmons, W. P., Jr., "Models Primarily Dependent on the Reynolds Number," a paper presented at the ASCE, Washington, D. C. Convention, October 1959

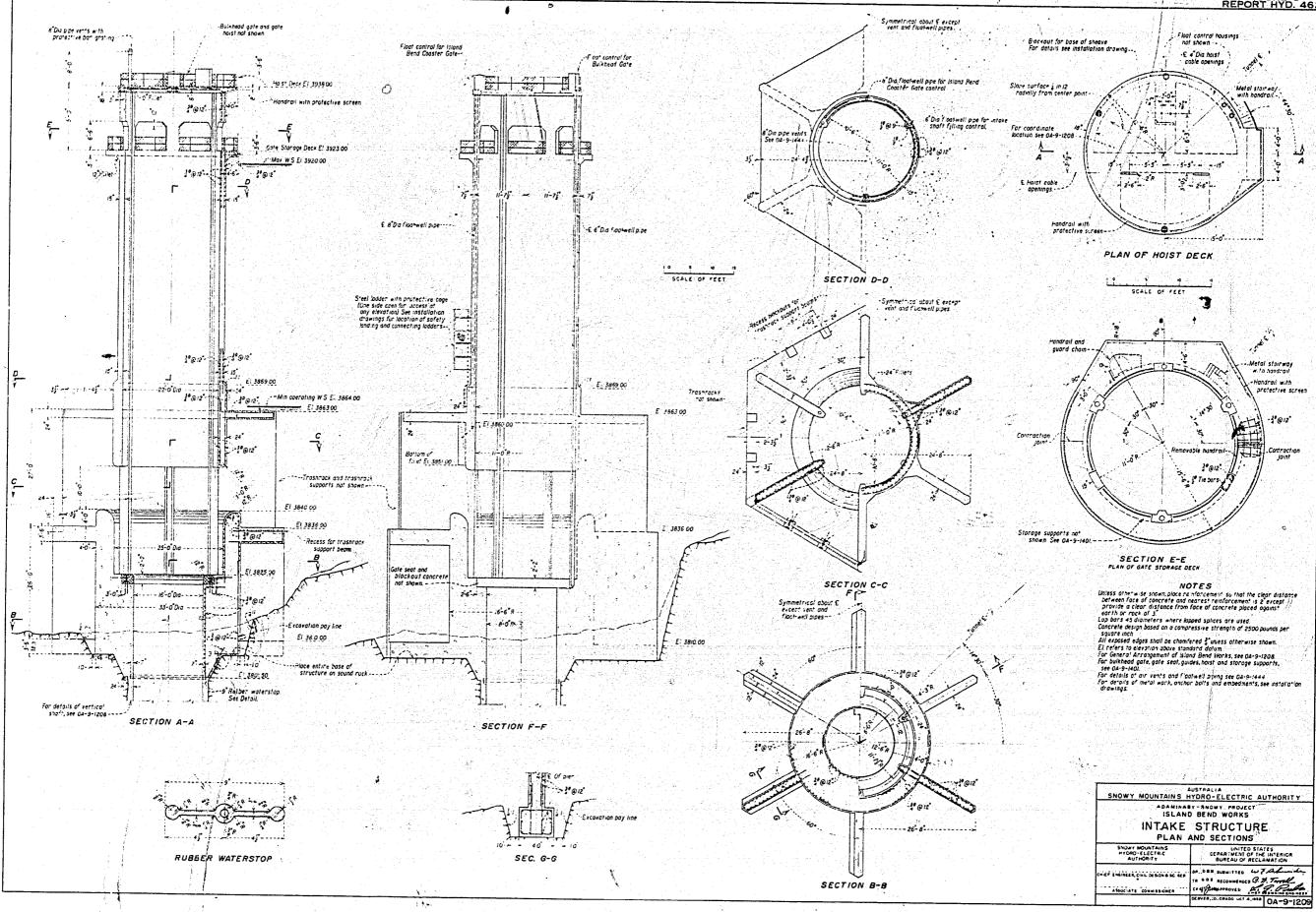


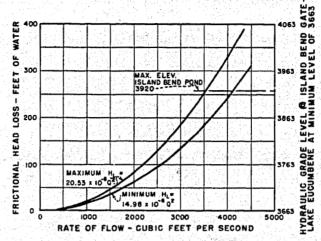




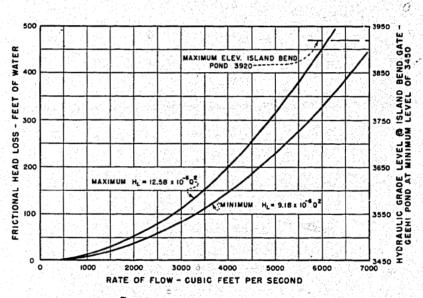








### A. EUCUMBENE - SNOWY TUNNEL



#### B. SNOWY - GEEHI TUNNEL

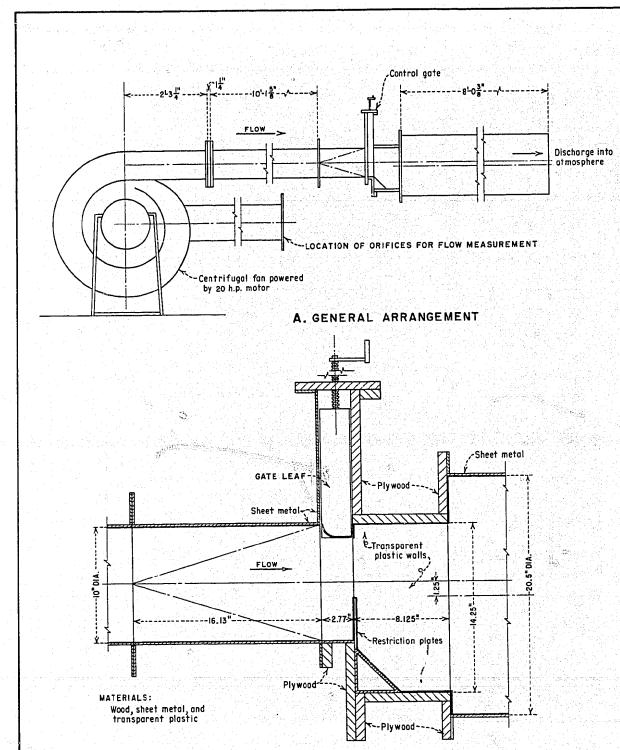
#### FRICTION FACTORS - MANNING EQUATION

MAXIMUM LOSS CONDITIONS
n=.029 fcr.70% of tunnel-rock
with lined invert D = 21.17
n=.014 for 30% of tunnel-concrete
lined D = 20.5

MINIMUM LOSS CONDITIONS n = .025 & n = .011

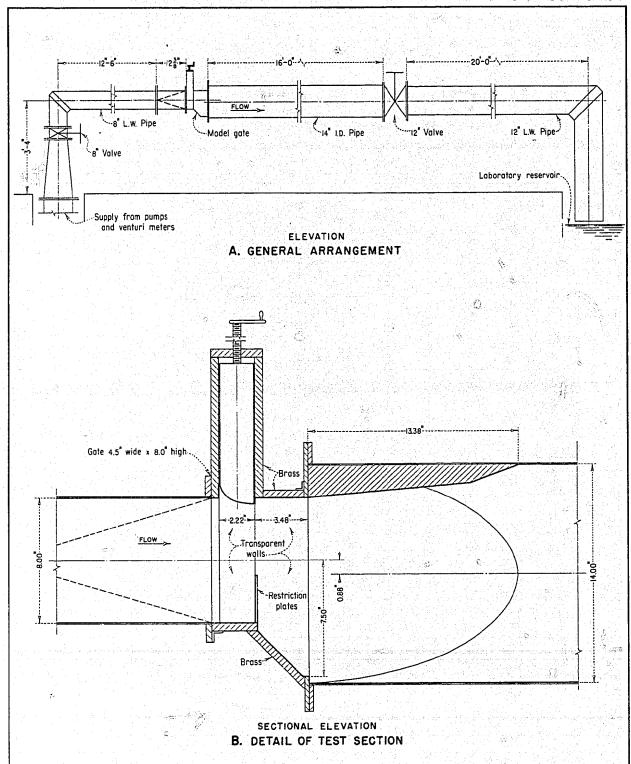
Note::Tunnel sizes from Dwg. DT-J-11

ISLAND BEND CONTROL GATE
COMPUTED TUNNEL FRICTION LOSSES, AND HYDRAULIC
GRADES AT ISLAND BEND GATE

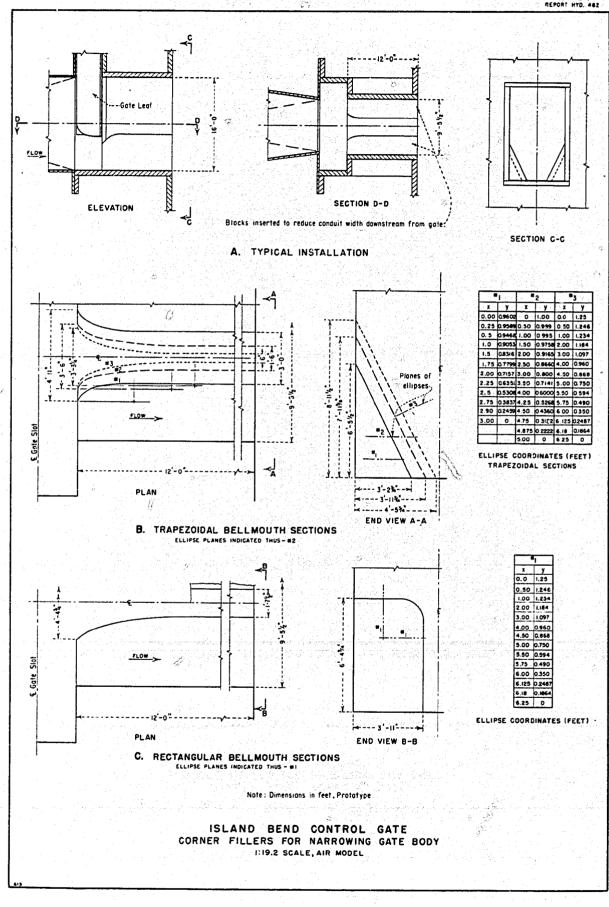


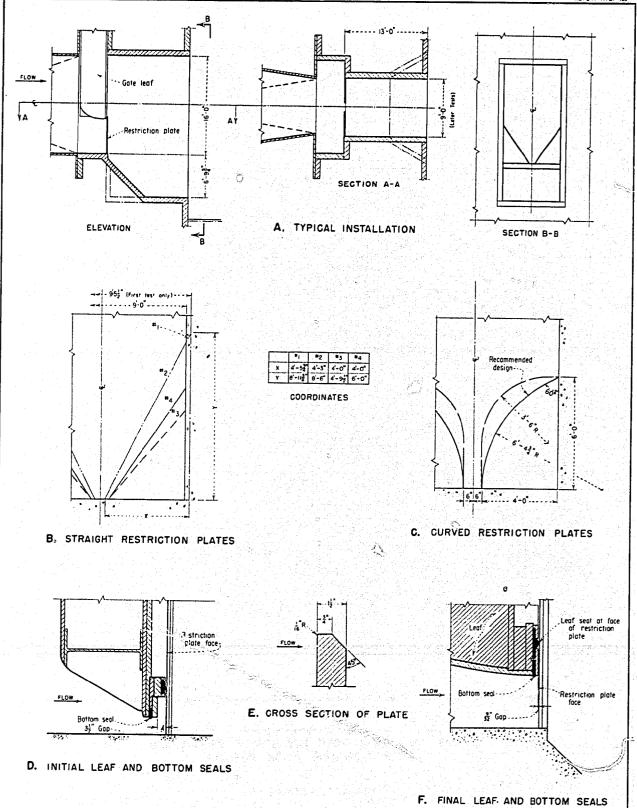
B. DETAIL OF GATE SECTION

ISLAND BEND CONTROL GATE
1:19-2 SCALE AIR MODEL



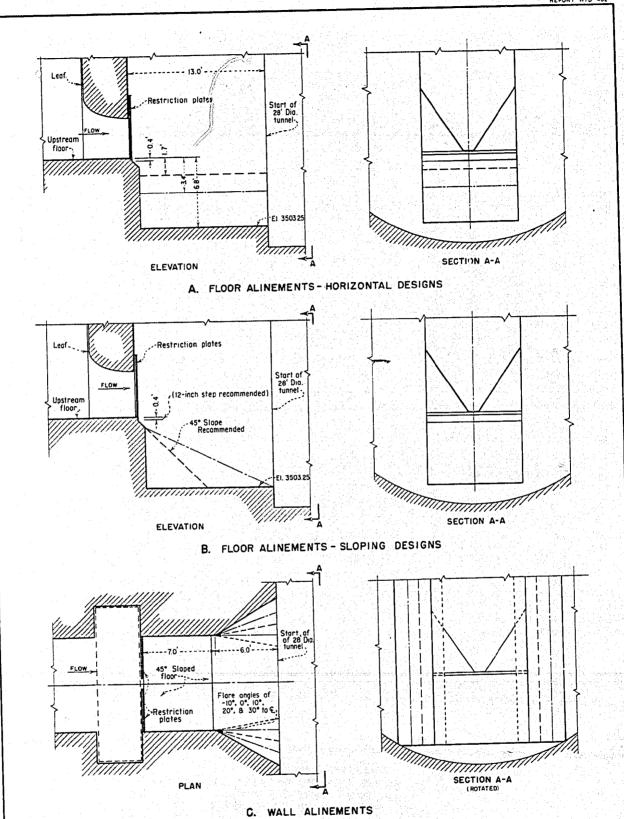
ISLAND BEND CONTROL GATE
1:24 SCALE HYDRAULIC MODEL





ISLAND BEND CONTROL GATE
RESTRICTION PLATES FOR NARROWING GATE BODY
1:19.2 SCALE, AIR MODEL

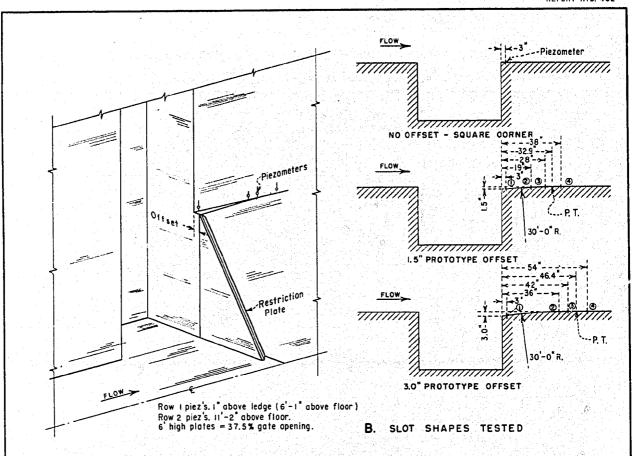
Note: Dimensions in feet, Prototype



Note: Dimensions in feet, Prototype

ISLAND BEND CONTROL GATE FLOOR AND WALL ALINEMENTS FROM GATE LEAF TO 28-FEET DIAMETER CONDUIT

1:19.2 SCALE, AIR MODEL



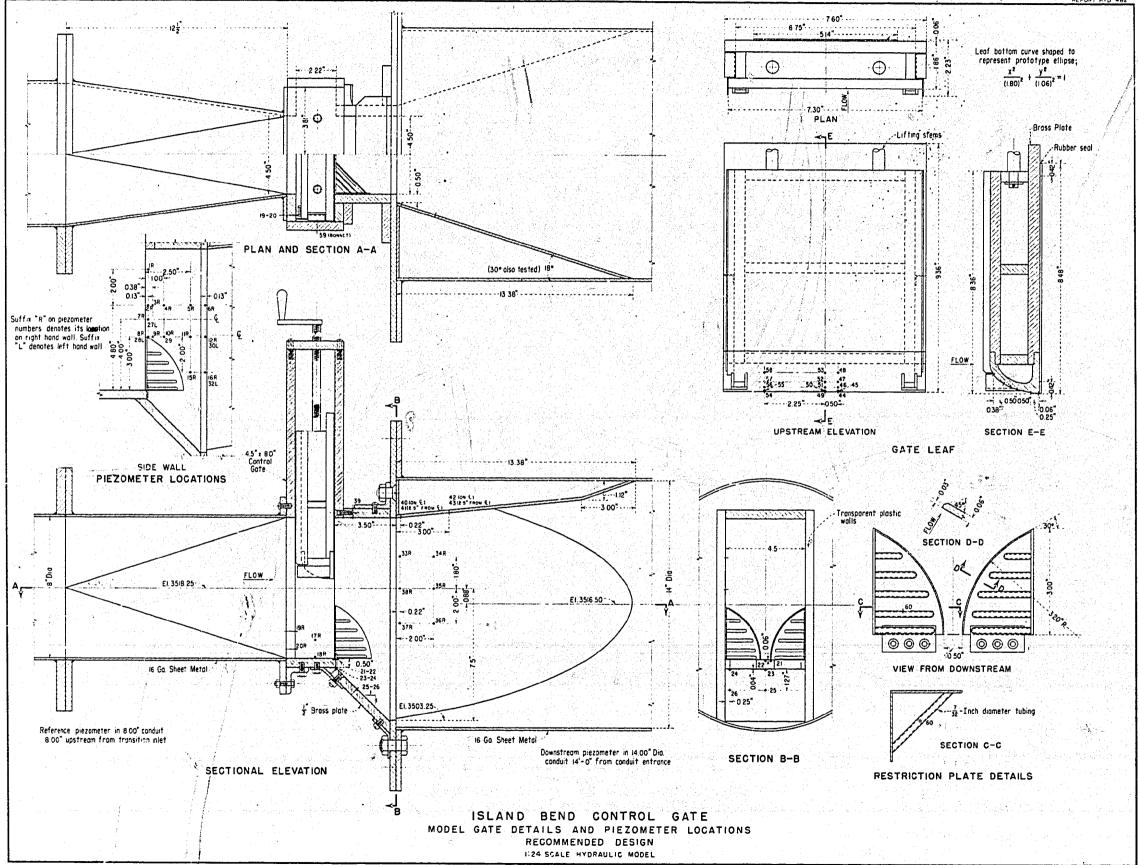
## A. OFFSET SLOT CORNER ABOVE RESTRICTION PLATES

SLOT		100		100	1 %	-14°		ATE (	DPENIN	IG 📜	1200		lad de	75.00	
	PIEZ.	32	.5 %	37	.5%	4	0%	4	5%	77	2.5 %	8	0%	10	0%
SHAPE	NO.	ROWI	ROW 2	ROW.I	ROW 2	ROW I	ROW 2	ROW I	ROW 2	ROWI	ROW 2	ROW I	ROW 2	ROW 1	ROW
	1	-12	-14	7	16	36	37	56	69	205	-18	100		280	257
NO	2	100	1 1 2	\$1.00 miles		PA.	A. Sa		1,150	655		0,000	-11-52	14,500	
OFFSET	3		1	100			45	4,41,	1000	State 1	800		44.77	100	160
	4	28.00	. 11	s, 1,7%		47.754	Taria.		siti u	astin ,	150	A. de V	50,00	2 200	
	1	-12	-14	7	16	36	37	62	69	205	- 18	7 4.0%		280	25
1.5 "	2	-7	-14	16	16	38	32	83	69	196	122		250	270	26
OFFSET	3	- 9	-14	16	16	36	37	83	67	189	124	100 40 40 4	1	265	26
	4	~ 9	-14	16	16	36	37	83	65	185	125		3,50	261	26
		-9	. <del>-</del> 14	14	18	18	39	68	76	201	19		1.5	267	26
3.0"	2	-9	-14	20	16	36	37	76	65	187	126			264	26
OFFSET	3	-9	-14	16_	16	36	37	76	65	189	124			264	26
1.00	4	-9	-14	16	16	34	37	76	65	187	126	1	40	264	26

C. PIEZOMETRIC PRESSURES - FEET OF WATER, PROTOTYPE, INCLUDING MINIMUM BACK PRESSURE (Tests Made With Elliptical Bottom on Gate Leaf)

Note: Data from 1:19.2 scale, air model

ISLAND BEND CONTROL GATE GATE SLOT DESIGNS AND PRESSURES



14

PIEZ	1	2	3	4	. 5	- 6	7	8	9	1	0	11	12	15	16	17	18	19	20	2	1 2	2	23	24	25	26	27	28	29	30	32	33	34	. 35	36	37	38	39	40	: 41	42	43	44	45	46	47	48	49	50	51	52	. 53	54	55	56	57	58	59	60	REF	· STR
100%	259	275	292	296	290	278	291	288	28	8 2	87 2	88	285	289	289	375	362	37	37	9 28	5 2	84 2	92	293	292	293	292	291	288	284	290	265	282	286	290	290	283	300	270	278	277	293	300	304	327	7 .33	8 34	0 29	0 30	3 325	33	8 33	3 315	329	334	353	344	340	286	372	3
90	201	232	265	273	270	263	267	274	27	4 2	74 2	74	272	274	277	363	363	376	38	0 27	8 2	70 2	78	280	280	280	267	274	274	274	276	258	270	274	276	276	264	246	280	267	275	276	243	249	28-	4 32	3 37	5 23	9 25	283	32	2 37	3 316	316	330	349	363	365	264	374	. 2
80	212	166	194	228	227	223	202	243	24	3 2	41 2	41	238	243	246	364	360	37.	38	0 24	7 2	38 2	48	249	249	250	204	249	245	240	246	210	235	242	245	245	238	506	237	234	251	243	221	228	263	3 298	34	5 216	22	260	29	7 34	291	3 302	321	346	353	374	240	377	2
70	174	148	152	165	173	175	111	198	19	8 1	96   1	96	191.	196	198	361	360	37	37	9 18	7, 18	38 2	203	205	205	205	117	201	196	194	200	165	186	192	200	199	184	168	174	183	189	197	188	199	243	3 278	32	8 184	1 19	239	27	8 32	2 271	3 308	304	325	328	375	195	381	2
60	111	93	99	105	106	110	87	127	12	8 1	27   1	27	123	128	132	361	364	389	38	1 12	0 11	4 1	35	138	139	139	.81	127	125	124	132	102	120	123	132	133	118	105	411.	114	: 28	126	141	160	223	3 26	31	5 134	16	219	26	3 30'	9 257	2 304	372	319	303	373	128	38€	5 1
50	32	32	33	31	32	35	30	41	46	4	6 4	7	47	50	53	381	391	39	40	0 3	в 2	9	58	61	62	.62	30	47	47	39	53	35	38	41	54	55	41	27	33	39	60	50	100	144	285	28	3 :33	2 .91	14	232	2 28	3 32	6 24	3 299	346	348	3 332	384	44	391	7
40	4	0	0	0	4	4	-2	8	9	13	8	7	5	8	14	380	388	38	39	2 1	2 -	4	21	25	26	26	I	:44.	111	8	311	4	4	6	15	16	7	0	6	4	21	i I	112	186	28	8 3.2	35	3 10	1 18	2 278	3 32	3 35	1 296	327	363	367	363	369	13	395	, 7
30	15	17	17	18	18	18	20	22	22	2	2	21	21	22	22	403	405	40	40	6 2	3 1	6	29	28	27	26	20	22	21	20	20	15	15	18	22	23	19	12	12	12	13	11.	186	280	359	373	39	1 17	3 26	3 52	2 37	7 38	8 391	8 395	399	397	393	373	24	400	5 7
20	20	24	24	23	23	23	26	28	28	2	6 8	26	26	30	30	407	408	40	40	7 3	0 3	0	27	26	24	24	26	26	26	24	29	55	5!	24	28	28	24	17	17	17	16	16	277	356	39	1 33	40	5 23	5 23	386	39	8 40	0 40	4 403	403	5 403	405	376	30	40	1 3
10	29	27	27	27	27	27	25	23	23	2	4 2	24	24	13	13	414	413	411	41	2   1	5 1	8	16	16	13	13	25	23	24	24	20	29	29	26	22	22	26	26	26	26	25	25	379	379	40	0 40	40	7 39	9 40	7 403	3 40	7 40	7 40	6 400	403	3 404	407	380	18	40	2 7
5								,						7 2	77.4										1.3	• 2		4.0	2.5					. 75			2.5				1.4.1	1 4 4,	386	392	399	9 40	1 : 40	3 30	8 37	398	3 40	1 40	2 40	1 40	402	2 407	402			402	2 3

PRESSURES - Island Bend Res. El. 3920

Geeni Pand Empty

30' Minimum Bock Pressu

OPENING PIE	2.	2	3	4	5	6	7	8	1	9	10	11	12	15	16	ī	7 1	3 19	9 2	20	21	22	23	24	25	26	27	28	29	30	32	33	34	35	36	37	38	39	40	41	42	43.	44	45	.46	47	48	49	50	51	52	53	54	55	56 :	57   5	8 59	60	REF	DISCH.
100%	62	7434	122	- 169	24	339	224	027	32	73 -	.277	270	310	25	925	6 +.7	796 +.	32 +.7	56 +.	848	302	324 -	.223	.215	220	-,214	22	237	-272	31	524	655	634	8 -293	250	250	338	121	-495	391	330	208	124	080	+.204	+.344	+.364	+ 246	-,081	+.182	+.344+	- 355 +	.114 +	.235 +	.296 +.	.531 +.4	22 +.30	55298	+.763	.6810
90	-1.02	0691	34	925	828	3837	132	824	52	45 -	.247	.247	-,270	24	722	21 +.6	92 +.6	93 +.8	26 +.	872 -	203	294 -	.203	193	188	188	324	254	246	6 - 25	223	341	429	225	223	22	359	550	181	325	240	- 229	578	-511	140	+.267	+.815	618	506	-152	+.256	+.797 +	.199 +	.191 +	.343 +.	.540 +.6	92 +.7	6359	+.808	6105
80	44	6822	59	-32	332	2936	152	2820	)42	201 -	.214	.215	-244	191	917	8 +.7	66 +.	39 + 8	43 +	.894	169 -	240 -	.162	-,154	149	147	513	154	188	22	3178	8 - 46	726	4212	183	183	242	499	249	- 274	- 136	198	378	319	044	+.237	+.620	-417	319	-,061	+.230	- 588 +	.236 +	.274 +	428 +	.628 +.6	81 +8	49224	+.874	4950
70	28	8437	7 -,41	33	729	528	65	6614	91	49 -	.159  -	:159	186	159	14	4 +.8	107 +.7	99 +.9	02 +.	.912	213 -	203 -	.115	107	103	107	619	127	158	- 17	135	534	122	1182	133	-,141	228	321	285	233	20,	155	202	142	+.119	+.321	+.612	.228	142	+.094	321	+.581 +	.320 +	496 +	-472 +.	.596 +.6	14 + 8	87 164	+.925	.3830
60	18	626	123	321	220	0819	29	00120	01	18 -	.120 -	.120	138	118	10	2 + 8	55 +.	65+.9	72 +	939	151 -	174 -	.087	077	072	071	-315	-121	129	13	510	022	6 - 15	1137	-100	09	158	213	188	[76	-,117	-,127	063	+.018	+.280	+438	+.667	.091	+.023	+.261	+.446 +	638 +	401 +	618 +	.691 +.	.679 + .6	11 +.90	5116	+.959	.2820
50	-,14	2140	137	14	14	113	-,14	6113	3(	)96 -	.095	.094	095	08	407	4 +.9	48 +.	77 +.9	81 +1.	.008	121	150 -	.058	-049	046	046	-,146	094	094	412	007	512	912	2 -,112	-,073	06	113	155	139	1:8	052	085	+.073	+.210	+.493	+.641	+.796	.043	+.216	+.482	-641	+.775 +	517+	692 +	.837 +	844 +.7	93 +,9	58103	+.980	.1899
40	-07	2083	308	208	107	0 -07	208	706	0 -0	58 -	.060	.064	067	06	004	14 + 9	54 + 9	74 +.9	73 +.	985 -	050	092 -	.024	015	012	011	-D79	051	05	106	05	307	0 - 07	2064	040	037	063	082	-065	072	026	079	+.224	+.424	+.703	+.793	+.880	.201	+.415	+.677	.799	+.875 +	.731 +	799 +	.907 +.	.919 +.9	08 +.9	24 - 045	5 +.993	.1125
30	04	034	403	403	203	3403	402	2602	1 -0	21 -	- 120.	.024	025	02	1 - 02	22 +1.0	05+1.0	11 +1.0	06+1.	.013 -	019 -	039	.003	.005	009	011	- 028	02	024	402	7 - 02	704	1 ~04	2034	02	020	-031	048	048	050	046	051	+420	+.675	+.887	+.926	+.973	.387	+.643	+.868	937	+.965 +	.992 +	985 +	.995 +.	.989 +.9	80 +.9	28016	+.999	.0533
20	-02	27016	016	018	03	2002	001	00	0-   6	07 -	.010	110.	012	+.00	1 0	+1,6	015 +1.	17+1.0	16 +1.	014	001-	.001	008	011	018	018	011	011	011	01	500	202	2202	4016	00	600	015	036	055	035	037	037	+.664	+.878	+.972	+986	1.009	·.553	+.816	+.959	990	+.995 +1	.005+	1003 +1	1.003 +1	.004 +1.0	09 +.9	3200	1+1.000	.0237
10	-00	3008	00	800	800	00	801	4 +01	8 +.0	18 +.	- 710.	.017	+.017	02	802	28 +1.0	32+1.0	29 +1.0	24 +1.	.026 -	040 -	.031 -	037	037	045	045	013	018	018	10	7  02	008	3 -00	3012	02	2023	012	010	-,011	011	013	013	+.938	+.940	+.996	H.015	+1.015	.993	1.013	1.005	1.015 +	1.015 +	.011 +	.995 +1	1.005 +1.	.005 +1.0	15 +.9	4003	1 +1,000	1600.
5									:   1			Çu i					PK_ 100		- 1 2	- 1 2	. "				1910	14.4		1 2 2			740	* - " - "			1			97,51		200	11	111	+.958	+.974	+.992	+.997	+1.003	+.748	+.930	+.989	+,997	1.000 +	.999 +	1.000 +	1,000+1	.000 +1.0	0.1+	00	+000	.0040

PRESSURE FACTORS

#### A. ISLAND BEND GATE - DOWNSTREAM TUNNEL FULL

PIEZ.	1	2	3	7	8	٥	lo.	::11:	.44-	-45	16	47	48	49	50	51	52	53	54	55	56	57	58	59	REF	DOWN-
100%	388	388	388	388	:369	390	390.	389	386	385	394	395	.385	386	386	393	392	384	387	387	388	387	385	385	401	399
90	360	374	377	375	375	378	380	382	368	365	370	382	403	366	364	368	382	403	388	382	376	380	392	392	400	392
80	369	366	369	366	367	373	374	375	366	360	361	367	387	364	360	359	367	386	384	382	372	371	378	397	401	385
70	366	352	362	356	356	364	366	366.	361	354	352	355	374	359	354	351	355	371	378	375	366	364	367	398	403	375
60	351	349	352	328	332	343	349	350	344	334	332	334	357	342	334	329	334	354	369	368	355	349	341	397	403	362
50	327	329	329	326	271	314	329	327	321	310	309	311	338	320	311	306	311	334	355	358	342	330	314	394	403	344
40	303	307	307	308	259	309	308	308	298	287	289	294	324	295	287	2.85	293	319	321	355	331	314	296	396	407	319
30	258	262	260	.263	264	264	263	263	256	246	258	271	.308	255	250	252	270	302	306	340	321	301	279	392	407	275
20	221	224	224	224	225	225	224	225	228	227	258	263	325	223	226	251	281	323	311	338	346	319	300	395	409	235
10	169	172	172	173	174	174	173	173	107	199	268	309	332	181	197	259	309	354	318	350	359	358	358	389	411	180
5	158	160	160	161	162	162	161	161	177	197	293	359	387	170	175	285	359	387	306	341	365	386	392	396	412	166

OPENING PIEZ.	, 1 .	2	3	7	8	9	10	1111	44	45	46	47	48	49	50	51	52	53	54	55	56	₹57	58	59	REF	DISCH.
100%	-775	-811	-789	-767	674	670	648	-696	938	969	357	278	-1,040	-938	-951	388	-449	-1.053	833	-877	~.811	850	969	982	+.211	1.238
90	-1.638	992	788	870	884	730	~,601	539	-1.249	-1.382	-1.119	495	+.543	-1517	-1.416	-1236	529	+.522	225	505	-799	604	027	+.010	+,392	1.087
80	- 556	-671	553	661	638	436	375	352	691	835	865	630	+.082	735	893	-929	643	+046	005	087	469	485	232	+.462	+.607	0.875
70	- 234	587	334	492	507	292	- 230	239	- 366	550	602	521	027	404	-555	641	516	083	+.083	+.014	237	293	189	+.664	+.764	0.725
60	-217	263	210	686	610	390	267	237	-366	555	-599	555	099	410	564	-655	567	-170	+.147	+.113	150	-261	426	+.701	+.814	0.602
50	-250	-218	-216	264	-1.078	-438	214	244	-,332	-496	-520	480	083	354	-492	560	490	154	+.156	+.206	034	-,210	-440	+.730	+.872	0.491
40	169	- 128	- 132	119	651	108	119	116	224	348	319	271	+.052	254	348	-362	276	-,001	+.015	+.353	+.128	+.050	244	+.829	+.951	0.382
30	-122	097	100	-089	-080	083	085	086	- 141	213	121	03i	+.7:38	149	181	167	039	+.197	+.224	+473	+.333	+.187.	+.032	+.848	+.961	0.277
20	078	065	065	060	055	056	059	060	039	044	+.129	+.270	+ 510	069	053	+.087	+.261	+.472	+ 427	+.582	+.571	+.474	+.365	+.905	+.984	0.177
10	-048	- 034	036	033	028	~.028	032	032	+ C32	+.083	+.378	+.555	- 740	+.005	+.073	+.340	+.555	+,749	+.593	+,733	+,770	+.769	+.767	+.901	+.997	0.085
5	-034	-,023	023.	019	015	016	020	020	+ ()44	+.127	+.514	+.781	+.896	+.015	+.118	+.481	+.781	+.896	+.565	+.708	+ .807	+.890	+.913	+.930	+.998	0.043
				1.					-		PRES	SUR	FAC	CTORS	}			3 7	7,	3.5						

(Approximating Eucumbene Gate)

B. GATE WITHOUT RESTRICTION PLATES - DOWNSTREAM TUNNEL FULL

OPENING 2 7 8 9 10 11 14 17 44 45 45 47 48 49 50 51 52 53 54 55 56 57 58 59 REF. -37 -40 -33 -26 -14 -5 -3 -25 -75 -79 61 193 -150 -80 -79 60 182 -138 -2 -65 -35 -57 -99 -87 202 -54 -65 -58 -44 -18 2 -9 89 -54 -13 -59 -4 240 -59 -93 -74 -3 242 170 102 -5 35 170 172 265 -17 -22 -11 -6 0 11 0 153 39 17 -79 -17 176 38 10 -94 -22 162 162 135 19 13 81 269 304 NO -7 -11 -9 -3 9 9 163 43 18 -82 -51 109 42 10 -99 -56 94 157 127 42 26 67 297 331 200 -26 -70 -82 -56 84 -18 - 51 -90 -58 68 172 156 73 42 1 310 355 60 50. 227 -37 -69 -57 -37 86 -52 -74 -73 -39 67 171 187 103 47 -20 318 373 246 -17 -40 -20 7 110 -30 -45 -35 2 94 172 213 143 88 26 324 388 40 277 -12 -18 24 62 156 -16 -20 10 58 140 149 240 194 144 89 332 398 30 317 -2 37 77 138 226 -8 3 64 134 215 205 254 260 221 185 329 404 20 395 5 43 188 301 363 -4 38 181 298 360 231 289 337 362 367 331 410

162. 2 7 8 9 10 11 14 17 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 DISCH -584 -590 -574 -556 -528 -505 -500 -554 -664 -665 -344 -023 -859 -688 -685 -346 -0349 -831 -497 -651 -5 9 -631 -733 -705 0.993 -.779 -.805 -.788 -.754 -.691 -.641 -.668 -.429 -.777 -.678 -.792 -.656 -.061 --. 83 | -.795 | -.768 | -.756 | -.742 | -.715 | -.742 | -.370 | -.647 | -.699 | -.933 | -.783 | -.312 | -.649 | -.718 | -.970 | -.796 | -.346 | -.346 | -.346 | -.412 | -.696 | -.709 | -.544 | -.085 | 0.709 -825 -835 -830 -815 -785 -899 -410 -703 -.764 -1008 -932 -.541 -.704 -.784 -1048 -943 -.578 -.424 -.498 -.704 -.743 -.644 -.083 0.607 -377 -930-1036 -1.065 -1002 - 660 -909 -990 -1086 -1007 - 699 -447 -486 - 687 - 763 - 864 - 110 0.513 60 -.357-1.001-1.077-1.049 -.993 -.701 -1.037-1.091 -1.087-1.005 -.747 -.492 -.454 -.658 -.796 -.959 -.135 0.423 50 40 -.346 -.987 -1.045 -.995 -.930 -.678 -1.020 -1.057 -1.032 -.941 -.716 -.527 -.428 -.597 -.733 -.883 -.156 0.340 No Flow 30 - 295 -1.000 -1.014 - 913 - 819 - 590 - 2011 -1.022 - 946 - 829 - 630 - 607 - 386 - 497 - 620 - 755 - 161 0 258 20 -.213 -.991 -.896 -.797 -.648 -.435 -1.005 -.978 -.829 -.659 -.462 -.485 -.367 -.351 -.447 -.534 -.184 0.170 -.101 | -.995 | -.894 | -.563 | -.400 | -.215 | -1.003 | -.925 | -.661 | -4.09 | -.232 | -.416 | -.261 | -.214 | -.209 | -.233 | -.179 | 0.086 10 -.036 -.988 -.896 -.565 -.266 -.115 -1.009 -.908 -.559 -.272 -.123 -.436 -.296 -.177 -.117 -.106 -.192 0.052

PRESSURE FACTORS

C. GATE WITHOUT RESTRICTION PLATES - FREE DISCHARGE

ISLAND BEND CONTROL GATE
AVERAGE PRESSURES AND PRESSURE FACTORS
1:24 SCALE HYDRAULIC MODEL

SUBMERGED DISCHARGE

Pressure Factor =  $\frac{h_x - h_2}{H_T - h_2}$ where  $h_x$  = head at piezometer,  $h_z$  = head in downstream conduit, and  $H_T$  = total head at reference station ahead of gate  $(h_0 + h_v)$ 

 $C_d = \frac{U}{A\sqrt{2g (H_1 - h_2)}}$ A = area of gate body - 9'-0" x 16'-0'

FREE DISCHARGE

Pressure Factor =  $\frac{h_1 - h_0}{h_0 + h_v}$ where  $h_x$  = head at piezometer,  $h_0$  = head at the reference station upstream from the gate and  $h_v$  = velocity head at the reference station

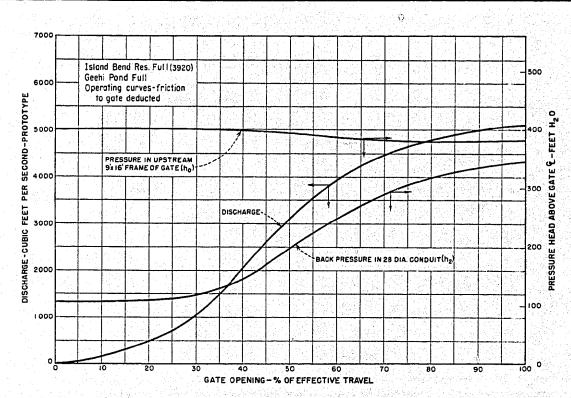
 $C_{d} = \frac{O}{A\sqrt{2g (h_0 + h_v)}}$ 

Computed pressures lower than about -30 feet, gage, are fictitious because no pressures lower than absolute zero con be obtained. In practice, cavitation will occur at -30 feet, and lower computed pressures indicate the cavitation severity.

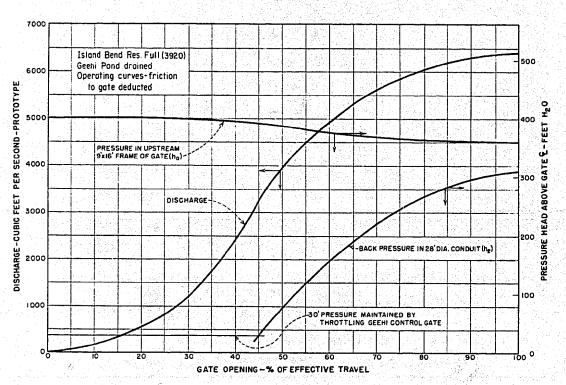
\*

49

13



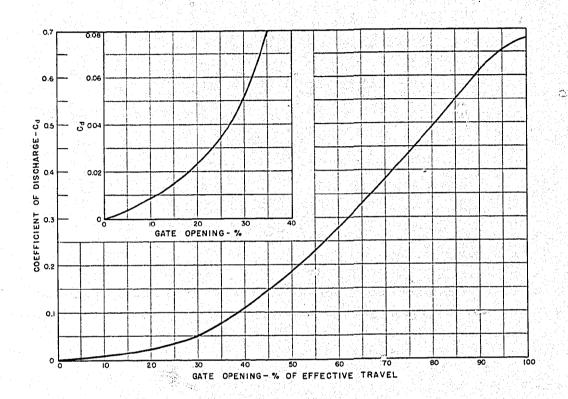
A. ISLAND BEND RESERVOIR FULL-GEEHI POND FULL



B. ISLAND BEND RESERVOIR FULL-GEEHI POND EMPTY

Note: Discharge from 1:24 scale hydraulic model

ISLAND BEND CONTROL GATE
HEAD, DISCHARGE, AND BACK PRESSURE AT CONTROL GATE
RECOMMENDED DESIGN

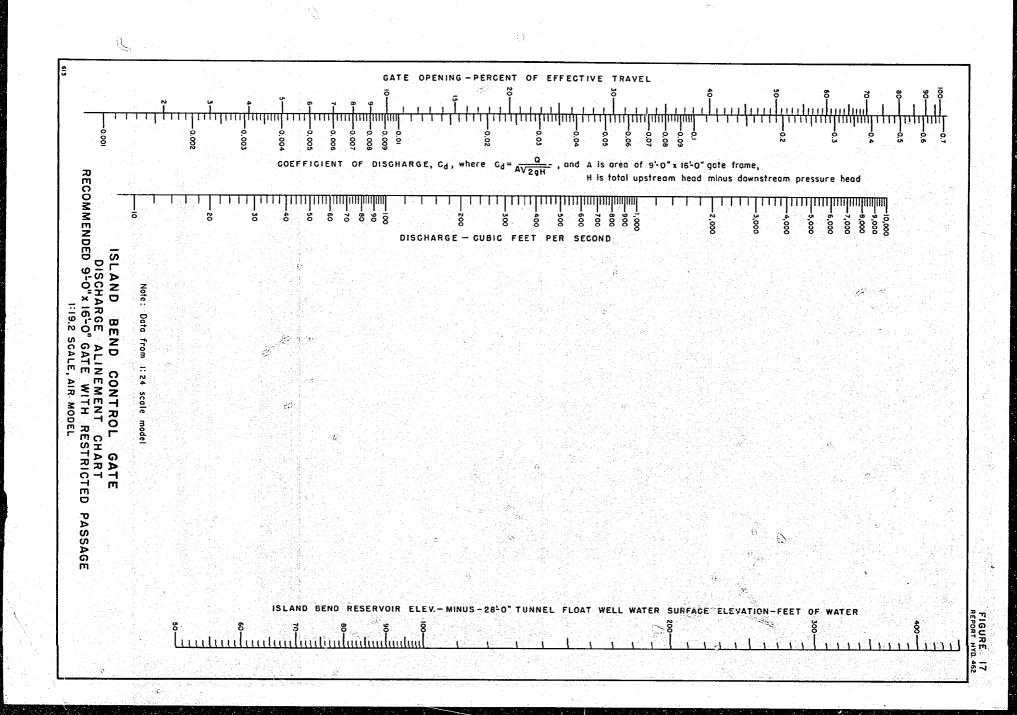


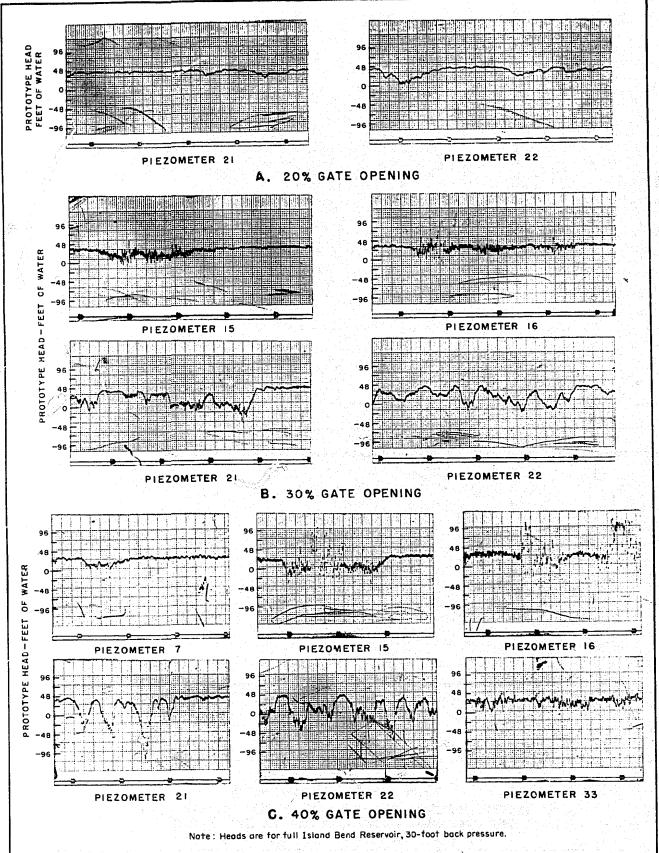
Where O = discharge, cfs
A = area of gate body passage, just upstream from leaf (8'-0" x I6'-0" prototype)
H = h<sub>0</sub> + h<sub>v</sub> - h<sub>2</sub>
h<sub>0</sub> = head at reference station in circular conduit 1 diameter upstream from transition to gate h<sub>v</sub> = velocity head at reference station h<sub>2</sub> = head in enlarged tunnel 100' downstream from leaf (prototype)

leaf (prototype)

Note: Data from 1:24 scale hydraulic model

ISLAND BEND CONTROL GATE COEFFICIENT OF DISCHARGE VS GATE OPENING RECOMMENDED DESIGN



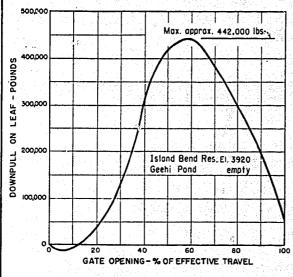


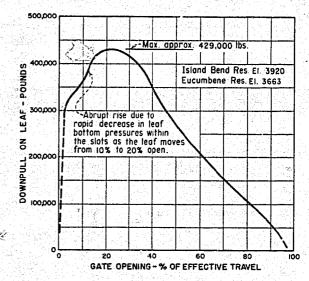
ISLAND BEND CONTROL GATE

PRESSURE CELL RECORDS OF MOST NEGATIVE AND
MOST FLUCTUATING PRESSURES-RECOMMENDED DESIGN

ISLAND BEND FULL AND GEEHI EMPTY
1:24 SCALE HYDRAULIC MODEL

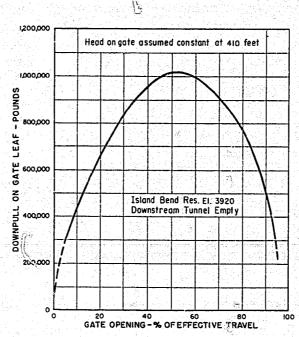
613





A. ISLAND BEND GATE Flow to Geehi

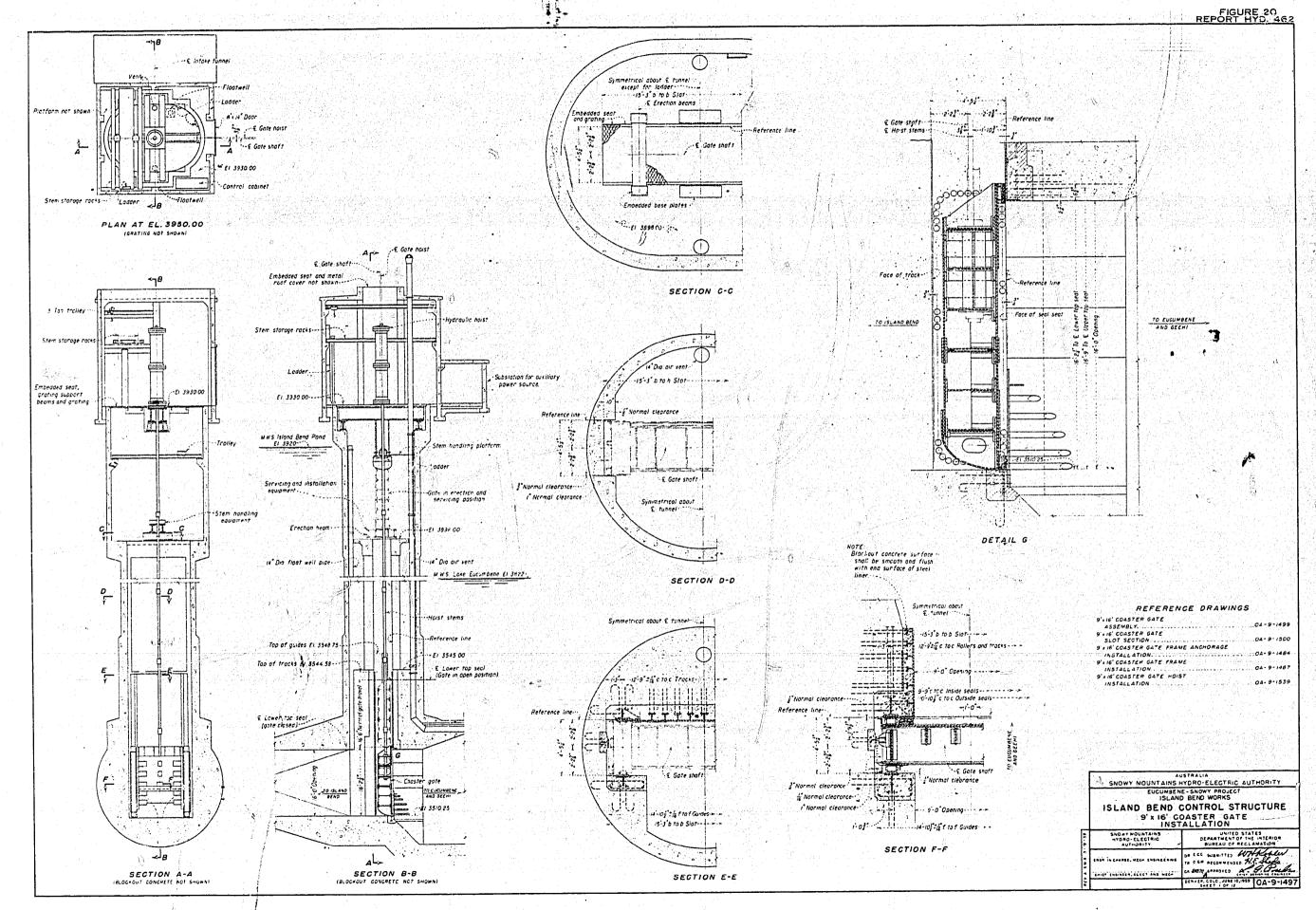




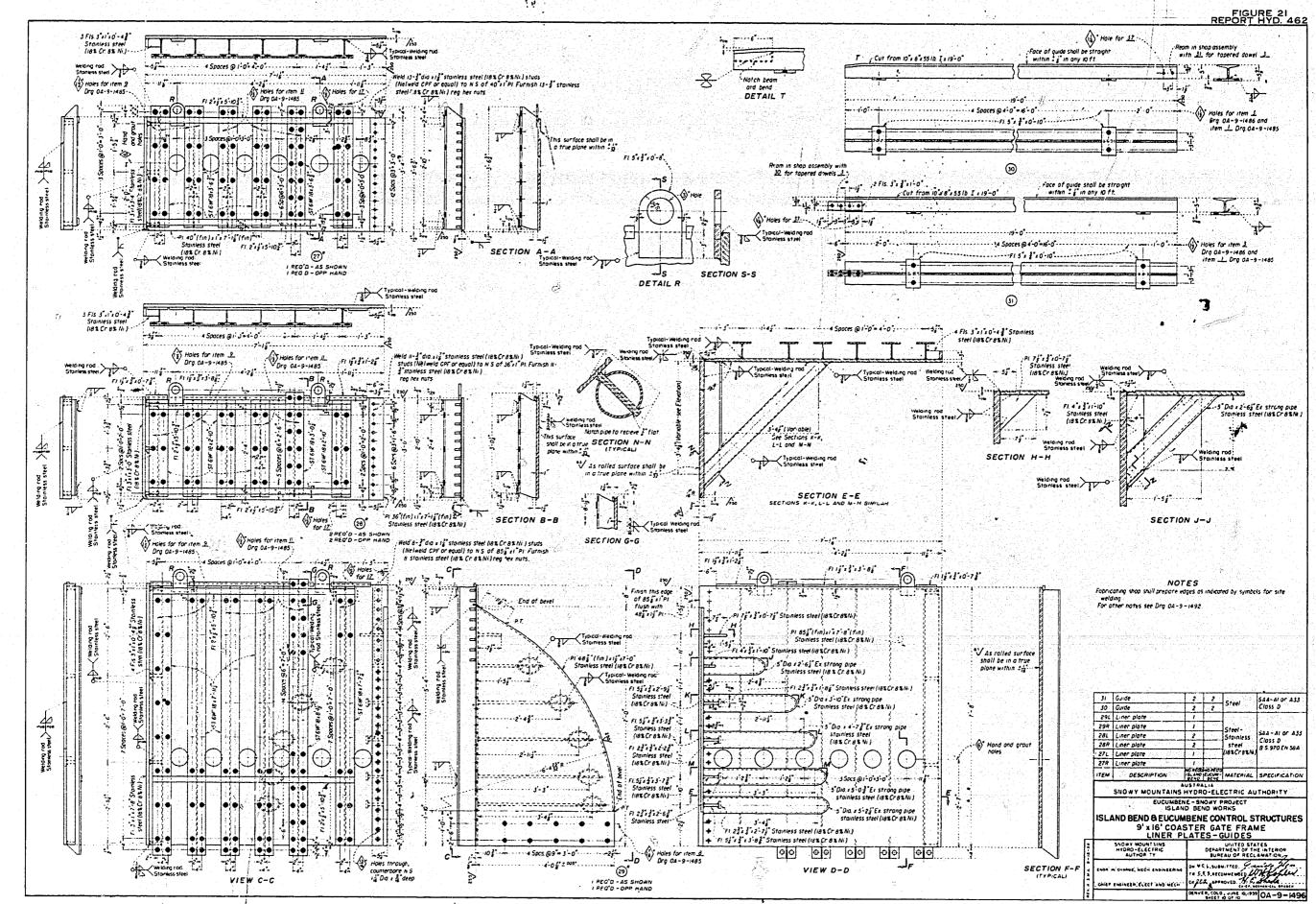
C. EUCUMBENE GATE - FREE DISCHARGE Flow to Eucumbene

Note: Computations based on pressure data from 1:24 scale hydraulic model

ISLAND BEND AND EUCUMBENE CONTROL GATES
HYDRAULIC DOWNPULL FORCES ON THE GATE LEAF
RECOMMENDED DESIGN



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